

Mechanical properties and debonding strength of Fabric Reinforced Cementitious Matrix (FRCM) systems for masonry strengthening

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ABSTRACT

Fabric Reinforced Cementitious Matrix (FRCM) composites are advanced cement-based materials often used for strengthening masonry or concrete structures. The system is usually composed of a dry grid of fibers embedded in a cementitious matrix enriched with short fibers.

An important parameter for designing the structural reinforcement is the tensile load-bearing capacity of FRCM composites. For their heterogeneity, FRCM composites show an interesting mechanical behavior in tension, that depends on the properties of the components and of the bonding strength. These values could be estimated with mechanical models but must be validated experimentally by means of proper testing campaigns.

In this work several FRCM materials made with different fiber grids were investigated. Four different types of fibers were considered: polyparaphenylene benzobisoxazole (PBO), carbon (C), glass (G) and PBO and glass (PBO-G) fibers and three different types of cementitious mortars.

The behavior of FRCM under tension and the influence of the bond properties between the dry textile and the inorganic matrix are studied developing an extensive experimental program that included the characterization both of the materials components and of the composites. A series of push-pull double lap tests and pull-off tests were performed to determine the bonding properties of FRCM composites applied to masonry structures.

The paper presents results and considerations that can provide background data for future recommendations for the use of FRCM systems in the rehabilitation of elements.

1. Introduction

Continuous fiber-reinforced materials with polymeric matrix (FRP) are nowadays widely used for strengthening and retrofitting existing masonry and concrete structures. The use of FRP strips as reinforcements instead of conventional methods is a suitable solution for the seismic upgrading.

These materials present many advantages: lightweight, speed of execution, good mechanical properties and good performance at failure [1–4]. They have also some drawbacks: poor behavior at high temperatures, relatively high cost of epoxy resins, lack of vapor permeability, incompatibility of resins and substrate materials, no reversibility of the installation [5]. The replacement of organic matrix with inorganic ones represents a solution to these drawbacks.

During the last two decades, the use of pre-fabricated cement-bonded fibers has increased. Such elements are used for many applications like cladding panels, exterior siding, pressure pipes,

tiles for roofs and floors. The reinforcement can be either made with short fibers (Fiber reinforced concrete, FRC) or with continuous fibers in a fabric form (Textile Reinforced Concrete, TRC) [6].

Fiber reinforced concrete is made with hydraulic cement, aggregates of various sizes and discontinuous short fibers. Since the early use of asbestos fibers, a wide variety of other fibers have been used with hydraulic cements: conventional fibers as steel and glass, new fibers as carbon or Kevlar and low modulus fibers as polypropylene or natural fibers. In TRC the fibers are usually pre impregnated with resin and this influences their mechanical properties that depend on the bond between the fibers and the matrix.

Fabric Reinforced Cementitious Matrix (FRCM) composites represent a particular type of TRC where dry-fibers in textile form are impregnated with a cementitious mortar enriched with short fibers. The multifilament yarns are usually disposed in grids and made of alkali-resistant AR glass, carbon or polymer fibers. The matrix is typically a grout system based on cement and a low dosage of dry organic polymers (less than 5% by weight). The organic compounds are necessary to ensure proper workability, setting time and mechanical properties [6].

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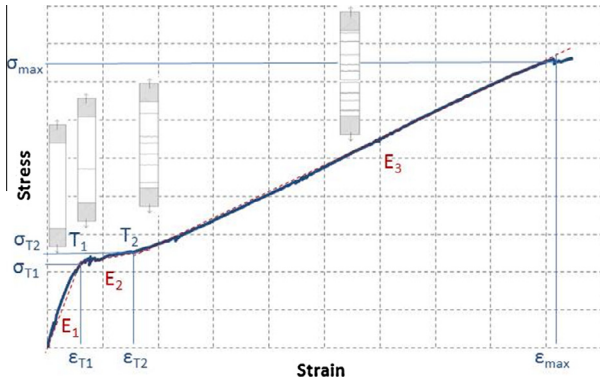


Fig. 1. Stress–strain behavior of FRCM composites subjected to tensile test.

FRCM composites are particularly indicated in the reinforcement of historical buildings, due to the higher compatibility with the substrates, vapor permeability and durability to external agents [7,8] and the major reversibility of the intervention. In fact the organic matrices penetrate into the substrate while the inorganic matrices cannot cause any damage. An overview of the use of FRCM can be found in [9–14]. To design the structural reinforcement both the values of the mechanical properties of the applied materials and their bonding resistance to the substrate are needed. These values may be determined only by appropriate experiments possibly made in specialized laboratories on an adequate number of specimens made with the same system of reinforcement and manufacturing techniques. The test procedures need to be defined in details and reported in guidelines or recommendations. A relevant work has been done in this direction by a RILEM Technical Committee (232-TDT).

Various set-ups for tensile tests on FRCM specimens have been developed [15,16]. The specimen production, dimension and shapes and load application may largely influence the stress–strain behavior of these materials. Hartig et al. [17] classified two types of

load application: “rigid load application” in which the main transfer mechanism between the specimen and the clamping is made with an adhesive in tension and shear and “soft clamping” with friction load transfer allowing for gradual load application.

This paper presents experimental results that can provide background data for future recommendations in this field. In this project specimens dimension and type of load application were decided in accordance with both the American code AC 434 [18] and the instruction provided by RILEM technical committee 232-TDT.

Another important aspect is the bond strength of the FRCM materials applied on masonry elements and the slippage phenomena at the interface mortar-textile and substrate-composite. The following failure modes could be identified during a push–pull shear test [36]: debonding at the substrate-composite interface, delamination of the composite at the matrix-textile interface and slipping of the fibers into the matrix. Many factors influence the bond strength of the FRCM system: the mechanical properties of textile and matrix, the mortar capability to penetrate into the grid and the friction between the filaments.

Several studies were developed to analyze these aspects for FRPs used in reinforcing concrete and masonry elements. On the contrary only few studies have been dedicated to FRCM systems [23,36,37]. In this paper the bonding behavior of the FRCM systems was investigated to analyze both the adherence between the reinforced system and the substrate and between the dry textile and the cementitious mortar. For this purpose, push–pull double lap tests and pull-off tests were carried out on different FRCM materials applied on clay bricks with different mechanical properties.

2. Mechanical behavior of FRCM in tension

A typical stress–strain behavior of FRCM under tensile test is represented in Fig. 1. The curve can be considered tri-linear: the first phase represents the uncracked state, where the slope of stress–strain curve reflects the elastic modulus of the matrix. The second phase corresponds to the formation of cracks. In this state

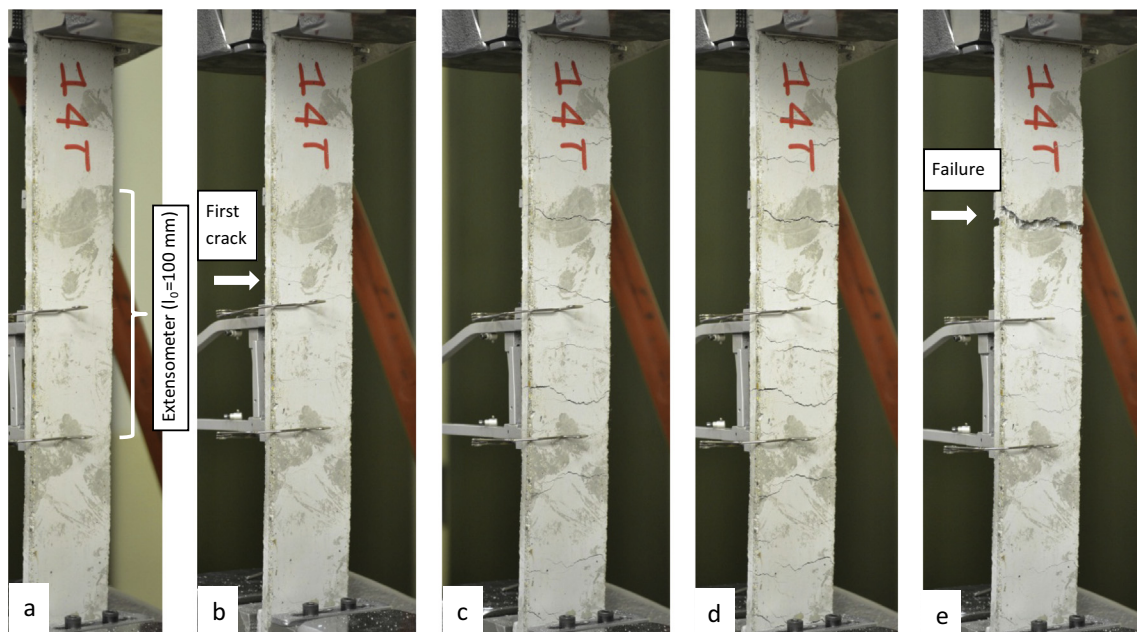


Fig. 2. Failure mode of FRCM composites subject to tensile test: (a) unloaded specimen; (b) first crack (point T_1 in Fig. 1); (c) multiple cracks in the mortar (point T_2 in Fig. 1); (d) fully cracked specimen; (e) ultimate failure.

there is a significant decrease of the stiffness and relatively fine cracks grow with the tensile load. The length and slope of this portion of the curve depend on the quality of the bond between textile and matrix and on the volume proportion of the fibers activated for the load transfer [8]. The geometry of the textile, as shown in Chapter 6, could be correlated with the location and the width of the cracks.

The third phase is the crack-widening region, where the existing cracks become wider up to the final failure that can be compared with the tensile strength of the textile. In few tests where the transition from second to third phase was not evident (point T₂), some extra cracks developed also in the third phase. In this phase the only resistant part in the composite is the fabric and, therefore, the slope of the curve should reflect the elastic modulus of the dry fibers. As showed in Fig. 1, the points T₁ and T₂ are defined by a slope change of the stress-strain curve and may be determined by the intersection of the lines tangent to each phase.

Fig. 2 shows the different phases of the test, the cracking of the specimens and the failure modes.

The main significant parameters are:

- Tensile stresses and strains in the transition point between two phases (point T₁ and T₂), σ_{T1} , σ_{T2} , ε_{T1} , ε_{T2} .
- Stiffness of the phases E₁, E₂, E₃.
- Ultimate tensile stress and strain, σ_u , ε_u .

These stresses are determined dividing the load by the transversal area of the fibers that are known. On the contrary the thickness of the matrix is quite variable and is *a priori* unknown.

As an alternative the tensile stress in the first phase can be calculated also dividing the load by the mortar area when the aim is to compare the elastic modulus with the one of the mortar and the

stress of the uncracked mortar in T₁ with the maximum tensile stress of the mortar.

In this work different types of FRCM materials were analyzed. For each typology a mechanical characterization was carried out for the components (cementitious mortar and fiber grid) and the composite.

3. Materials

The studied FRCM systems consist of two main elements, a cementitious mortar enriched with a low dosage of dry polymers and a fiber grid. In this study different types of fiber grids were investigated: polyparaphenylene benzobisoxazole (“PBO-1 fiber” and “PBO-2 fiber”), glass (“G fiber”), carbon (“C fiber”) and an hybrid grid composed of PBO and glass fibers (“PBO-G fibers”). Each type of fiber was correlated with a specific mortar. The corresponding system are denominated “PBO1-FRCM”; “PBO2-FRCM”; “G-FRCM”; “C-FRCM”; “PBO-G-FRCM”.

3.1. Textile reinforcements

The “PBO-1” fiber unbalanced net is made of 10 mm and 20 mm spaced rovings (Fig. 3a). The free space between rovings is roughly 5 mm and 15 mm respectively, and the equivalent thickness in the two fibers directions is 0.046 mm and 0.011 mm respectively.

The “PBO-2” fiber balanced net is made of 12 mm spaced rovings. The free space between rovings is roughly 10 mm, and the equivalent thickness is 0.014 mm (Fig. 3b).

The glass fiber unbalanced net presents a coating in Styrene Butadiene Rubber (SBR) that provides a resistance to alkaline environments. The free space between yarns is 17 × 12 mm (Fig. 3c).

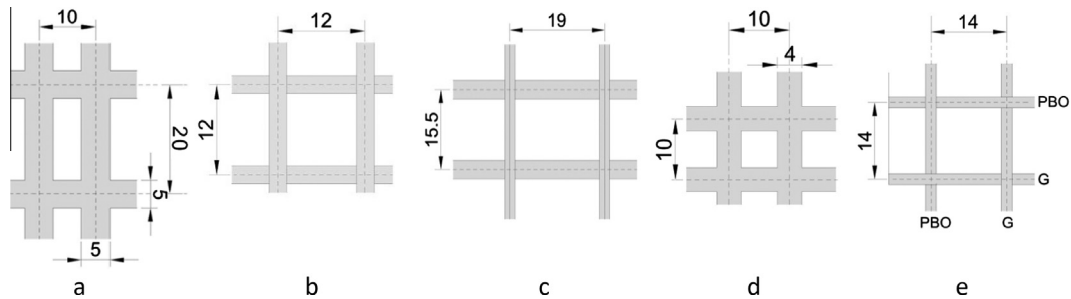


Fig. 3. Geometrical dimensions: (a) PBO-1 fiber mesh; (b) PBO-2 fiber mesh; (c) Glass fiber mesh; (d) Carbon fiber mesh; (e) PBO-G fiber mesh.

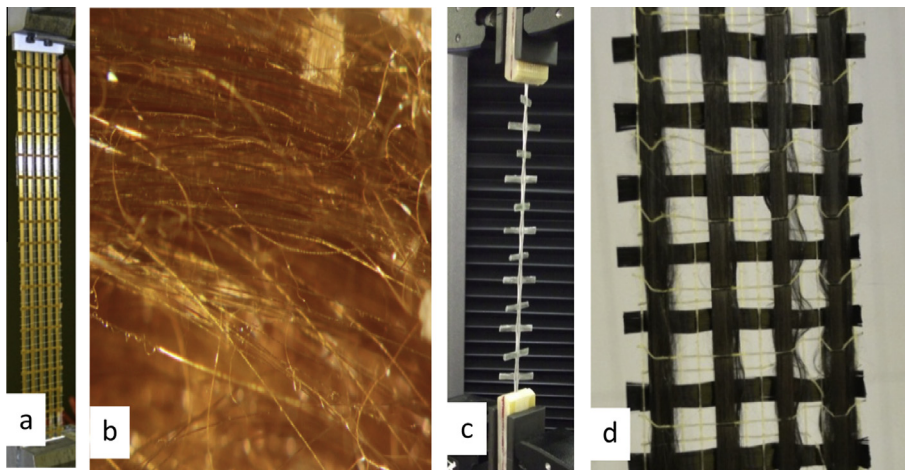


Fig. 4. (a) Test set-up, PBO mesh; (b) micro-failure of the filaments of PBO; (c) Test set-up for a single yarn of glass fibers; (d) Failure of the rovings in the carbon mesh.

The carbon fiber is a balanced net with fiber rovings disposed in two orthogonal directions at a nominal spacing of 10 mm (Fig. 3d). The equivalent fiber thickness is 0.047 mm in both directions.

The “PBO-G” net is composed of PBO and glass fibers rovings. The free space between rovings is 14 mm and the equivalent thickness is 0.0064 mm (Fig. 3e).

In all the geometries, excluding the glass textile, the longitudinal and transversal yarns are not glued together, but woven and fixed with small nylon fibers (Fig. 4a and d).

Tensile tests of the dry fibers were performed according to EN ISO 10618/2005 [20] on a single roving in warp and weft directions and on a grid strip of width 4 and 5 cm in the warp direction. Tests were carried out using different testing machines with maximum load capacities of 2 kN and 100 kN and an extensometer with base length equal to 50 mm. In order to avoid local damage in the specimens during the tensile tests, special tabs of fiberglass were bonded using epoxy resin at the ends of the specimens. The tabs presented a width equal to the sample and a length equal to 60 mm.

The experimental results are summarized in Tables 1 and 2. The failure mode shows a rupture of some filaments without a complete failure of the roving. Fig. 4 shows the test set-up and the failure modes.

Different values of the ultimate strength were determined in the case of PBO-1 fibers. The main reason of the difference between the single roving and the grid is certainly due to an irregular distribution of the stresses in the four yarns. The different tensions in each roving are not easy to determine experimentally as even the tensile test on dry fibers is a difficult task influenced by the number of fibers in each yarn and their distribution. Tabs made with GFRP were used to distribute the stresses in the grids but the irregularities in the load distribution are difficult to avoid. The problem was limited to PBO-1 fibers while the other results could be considered satisfactory.

3.2. Cementitious matrix

Three types of mortars were considered. Table 3 shows the main mechanical properties and the corresponding experimental standard [21,34,35]. Some of the values were verified by the authors while others were taken from the technical data-sheets provided by the producer. The technical data sheets should give indications about the possible types of fibers that can be coupled with a mortar. The properties of the mortars are usually the result of long and deep research activities to find the best coupling mainly in terms of bonding properties and therefore some of the constitutive components of the mortar may be confidential.

4. Specimen preparation and test set up for tensile tests

4.1. Specimen geometry and preparation

In order to investigate the mechanical behavior of FRCM systems, tensile tests were carried out on several series of specimens. In the following part the test set-up is described and the main results of the experimental program are reported.

Tensile coupons were made in a flat mold by applying a first layer of cementitious mortar (5 mm), the fiber mesh and a second layer of cementitious mortar (5 mm). The curing lasted 28 days. In the curing phase is important to pay attention to the possibility that some micro-cracks develops due to a non-homogeneous shrinkage.

The coupons (Fig. 6) were all rectangular (nominal size 400 × 40 × 10 mm). Table 4 shows the variability of the specimen transversal area due to the non-constant thickness. This is a very important parameter in the following analysis of the first phase

Table 1
Tensile tests on single dry roving.

Material	Tensile tests	Cross section area (mm ²)	# tests	Average failure stress (MPa)	CoV (%)	Elastic modulus (GPa)	CoV (%)
PBO-1 fiber	Roving in the warp direction	0.41	6	3900	3.2	215.9	20.8
	Roving in the weft direction	0.21	2	3430	-	276.6	-
PBO-2 fiber	Roving	0.22	3	3175	2.4	-	-
Glass fiber	Yarn in the warp direction	0.90	5	1233	2.7	55.6	30.5
	Yarn in the weft direction	0.92	5	1120	1.7	60.5	28.2
Carbon fiber	Roving	0.42	3	1944	14.9	203.0	9.8

Table 2
Tensile tests on dry grid strip.

Material	Tensile tests	# tests	Average failure stress (MPa)	CoV (%)
PBO-1 fibers	Grid strip of width 4 cm (4 rovings)	4	3397	7.3
	Grid strip of width 5 cm (5 rovings)	6	3395	11.7
Glass fibers	Grid strip of width 5 cm (3 yarns in the warp direction)	5	1121	1.3
Carbon fibers	Grid strip of width 4 cm (4 rovings in the warp direction)	3	1913	10.4
PBO-G fibers	Grid strip of width 4 cm (2 PBO rovings and 1 Glass roving)	3	2996	11.9

Table 3
Mechanical properties of the mortars.

Material	Tensile test (MPa)		Compressive test (MPa)		Flexural strength (MPa)		Elastic modulus (GPa)
	EN 12390-6		EN 1015-11		EN 1015-11		
	Tensile strength	CoV (%)	Compressive strength	CoV (%)	Compressive strength	CoV (%)	
Mortar used with PBO fibers	4.75 (7)	4.05	>15 (data sheet)		>2 (data sheet)		>6 (data sheet)
Mortar used with glass fibers	-	-	27.13 (7)	4.10	8.38 (14)	13.15	8 (data sheet)
Mortar used with carbon fibers	-	-	>20 (data sheet)		3.5 (data sheet)		>7 (data sheet)

Note: Within brackets # of tested samples or font of the data.

Table 4
Average specimens size.

Material	Section max (mm ²)	Section min (mm ²)	Section average (mm ²)	CoV (%)	Volumetric reinforcement ratio
PBO1-FRCM	478.1	263.9	382.9	16.9	0.0041
G-FRCM	450.5	380.2	410.6	4.1	0.0045
C-FRCM	367.3	300.2	340.7	6.4	0.0042

of the tensile tests. Due to the difficulties in defining a specific volumetric reinforcement ratio for each specimen, only one nominal volumetric reinforcement ratio was adopted for each composite material making reference to the nominal mortar area (400 mm²). The values of the volumetric reinforcement ratio are showed in Table 4.

Another important aspect for FRCM composites is the behavior of multi layers FRCM composites. These are not examined in this work. Only few specimens were prepared to analyze the specific problem of the overlapping of the textile in a single layer specimen. The technique of overlapping is often adopted in the application of FRCM to strengthen large structural elements. These samples were realized with the same technique and with a similar geometry. Only preliminary test were performed but we can conclude that if the overlapping is located in a central part of the specimen, with a length of at least 100 mm the failure mode is similar to that of original samples without grid overlapping but in the third phase slippage phenomena were highlighted between the fibers and the matrix. This problem should be studied in future projects.

4.2. Clamping

Tensile tests were carried out according to Annex A of AC 434 [17], but with a different gripping mechanism. The US Standards recommend a clamping method with Clevis-type grips and one of the two grips shall allow for rotation in two perpendicular planes to avoid the negative effects of possible eccentricity and misalignments [19]. In this project an alternative system was studied and adopted after a comprehensive comparison of the two methods. In the adopted clamping system the two extremes of the specimens were fixed into the grips of a standard testing machine but the lower grip allowed for torsional rotation. In this case the clamps can produce high compressive stresses at the end of the specimens. For this reason fiber reinforced tabs (dimensions 60 × 40 × 2 mm) were applied using epoxy resins after sand blast-ing of the extremes. After several tests for comparison it was concluded that this clamping method was satisfactory and preferable. Furthermore it gives the possibility to reach the ultimate stress of the textile reinforcement and explore the third phase of the stress-strain plot (see Fig. 1) limiting the slipping of the fibers. On the contrary the US standard foresee a bilinear behavior of the composite limiting the analysis to the first two phases. An example of the results obtained using the two methods of clamping is reported in Fig. 5. The longer extension of the curve obtained with the present clamping system is evident.

4.3. Instrumentation

A testing machine with load capacity of 100 kN was used with displacement control at a rate of 0.1 mm/min in the first phase. After the cracking, the speed was increased to 0.5 mm/min. Various methods have been used to measure the strains in TRC and FRCM [19]. Strain gauges are inadequate since provide only local information. Extensometers are ideal to assess the alignment of the specimen and to avoid possible out-of-plane bending moment. The optimum set up would include four LVDTs placed on the opposite sides of the specimen. Photogrammetry can be another refined

method that provides a complete overview of the crack formation in the specimen. This would be the ideal system of measurement but it becomes burdensome for large series of tests. The US Standards AC 434 [17] suggest to use an extensometer with a minimum gauge length of 50 mm that shall be adequate to at least include one significant crack. In this study the deformations were measured using an extensometer with a gauge length of 100 mm positioned in the central area of the specimens (Fig. 7). Being the dimension of the specimens 400 × 40 × 10 mm, the distance

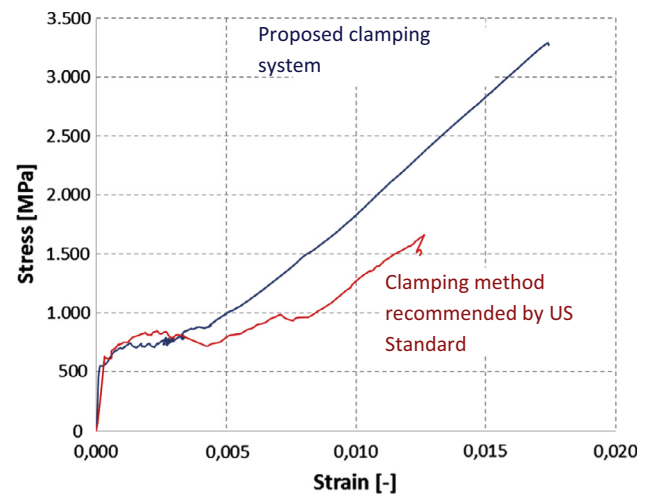


Fig. 5. Stress-Strain behavior: comparison between two clamping methods.

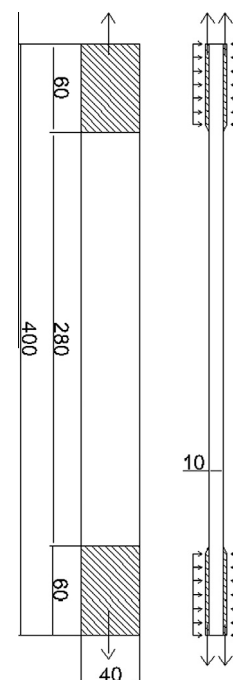


Fig. 6. FRCM specimens size.



Fig. 7. Tensile test set-up.

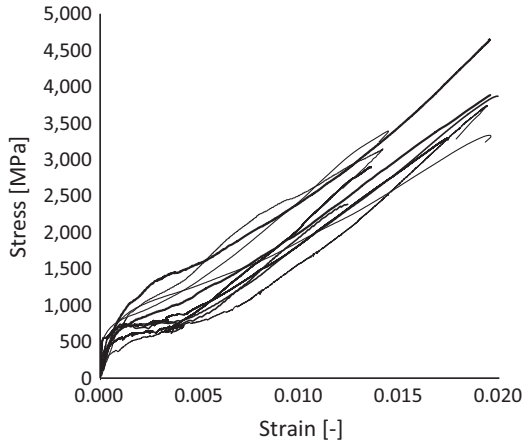


Fig. 8. Stress-strain curves for PBO-FRCM specimens.

between the two grips of the testing machine was 280 mm. Therefore the extensometer gauge length of 100 mm covered about 1/3 of the free surface of the specimen giving an adequate measurement of the strain field. This was checked by means of a detailed comparison with the displacement of the clamps.

5. Tensile test: experimental results

The results of tensile tests are here presented for the following materials: PBO1-FRCM, G-FRCM and C-FRCM.

Table 5
Tensile tests results on PBO-FRCM.

	E_1 (GPa)	E_2 (GPa)	E_3 (GPa)	σ_{t1} (MPa)	σ_{t2} (MPa)	σ_u (MPa)	ε_{t1} (%)	ε_{t2} (%)	ε_u (%)	E_1^* (GPa)	σ_{t1}^* (MPa)
Average	1180.8	75.7	215.8	890.2	1099.6	3316.4	0.08	0.50	1.69	4.84	3.65
Min value	902.6	38.8	189.2	509.8	551.1	2485.1	0.04	0.24	1.27	3.70	2.09
Max value	1612.8	122.4	250.0	1119.5	1640.6	4669.4	0.13	0.81	2.00	6.61	4.59
CoV [%]	19.6	33.1	9.2	15.2	12.6	14.0	30.90	33.70	18.20	19.60	15.20

5.1. PBO1-FRCM

Tensile tests on PBO1-FRCM specimens show a tri-linear behavior as in Fig. 1. In the first linear branch the specimen is uncracked and the slope reflects the elastic modulus of the cementitious mortar. Once the tensile strength of the mortar is reached (point T_1) the force is transferred to the textile (second phase). In the third phase (starting from point T_2) only the contribution of the textile is noticeable and the slope reflects the elastic modulus of the dry fibers.

As expected, the results show a large variability in particular for the localization of points T_1 and T_2 . This phenomenon is mainly caused by two parameters: the non-regular dimensions of the specimen section (see Table 4) and the location of the first crack with respect to the extensometer [38]. As described in Chapter 2, the extensometer had a length of 100 mm and was located in the central part of the specimens. Therefore the cracks that appeared outside this interval could not be recorded.

A series of 23 tensile tests made with PBO-FRCM was tested. Fig. 8 shows the stress-strain behavior of some specimens. In Table 5 a summary of the results is reported. The parameters were derived dividing the loads by the section of the fabric. Only for the first phases, in which the mortar is un-cracked, tensile stress (σ_{t1}^*) and elastic modulus (E_1^*) were also referred to the composite cross section in order to compare the cracking tensile stress and the elastic modulus with the mortar properties. These values are reported in the last two columns in Table 5. The stress in the mortar at point T_1 is 3.65 MPa that is lower but comparable to the nominal maximum tensile strength of the mortar (4.27 MPa).

The elastic modulus of the third phase (215.7 GPa) is very similar to the elastic modulus of the dry PBO textile (216 GPa). Fig. 9 shows a comparison between the tensile test on PBO-FRCM with 4 rovings, and the dry PBO textile with 1, 4 and 5 rovings.

The experimental results show a quite large variability, in particular for the elastic modulus of the second phases and the deformations corresponding to first and second points of transition. These variability could be caused by several factors: the irregularities of the cross section of the specimens, the

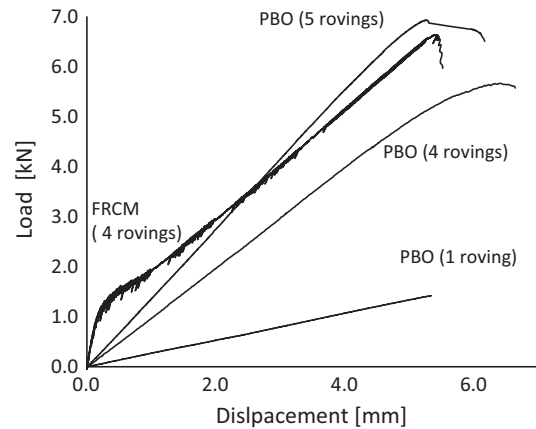


Fig. 9. Force-displacement curves for PBO-FRCM and dry PBO fibers (1 roving, 4 and 5 rovings).

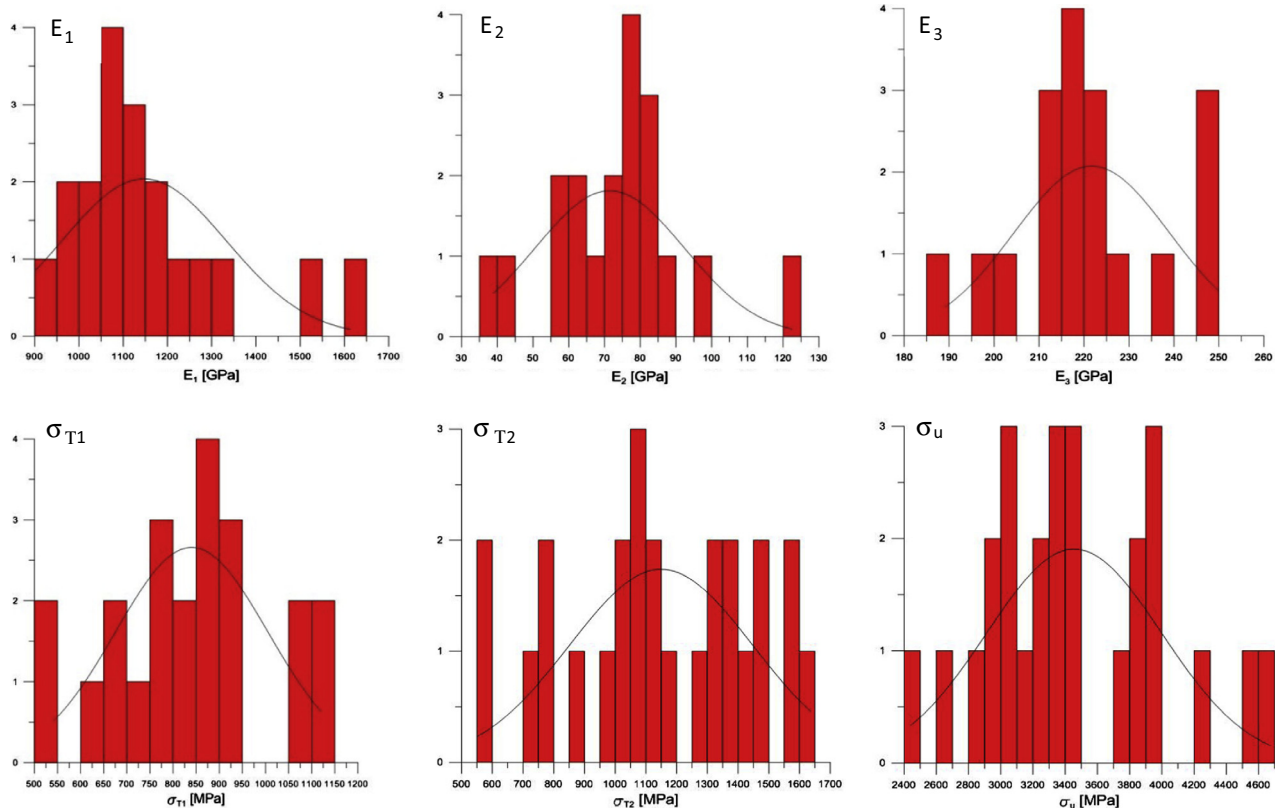


Fig. 10. PBO-FRCM: Statistical analysis of E1, E2 and E3, and stress in point T1, T2 and ultimate stress.

presence of micro-cracks invisible to the naked eye, the irregular position of the textile in the thickness and the localization of the first cracks with respect to the extensometer.

Fig. 10 shows a statistical analysis of these parameters.

Three further tensile tests were carried out applying cyclic loadings. The test set-up was the same as for monotonic tests while the cycles were applied in load steps of 0.2 kN and displacement steps

equal to 0.1 mm in the cracking phase. Fig. 11 shows a stress-strain curve compared with a correspondent monotonic curve. A good correlation between the two curves is evident even if, as expected, the maximum load is lower than the average failure load

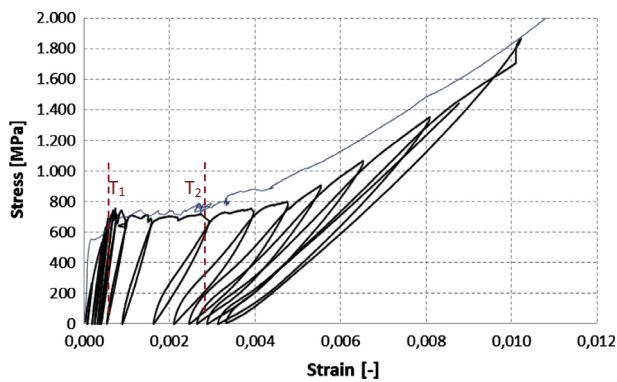


Fig. 11. Stress-strain curves, comparison between monotonic test and cycling test.

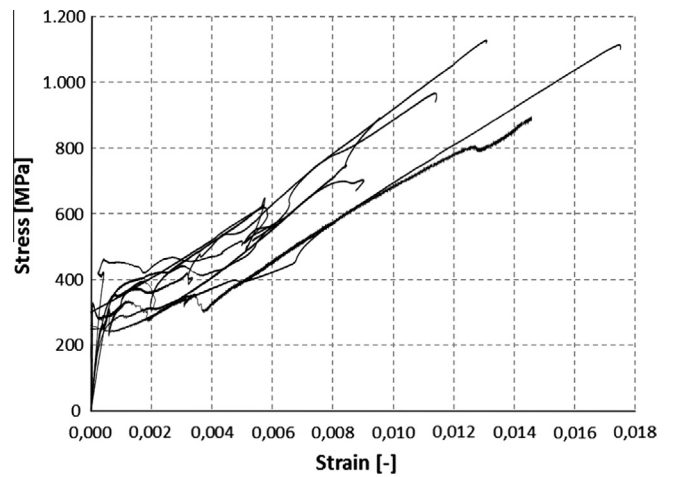


Fig. 12. Stress-strain curves for G-FRCM specimens.

Table 6
Load-Unload tests results on PBO1-FRCM.

Sample	Stress (MPa)		Residual displacement (mm)		Residual strain (%)		Average Crack residual dimension (mm)	
	T ₂	Collapse	T ₂	Collapse	T ₂	Collapse	T ₂	Collapse
PBO1-FRCM1 load/unload	687.2	1838.7	0.53	1.20	0.19	0.38	0.088	0.12
PBO1-FRCM2 load/unload	819.1	1535.7	0.47	1.57	0.17	0.27	0.067	0.17
PBO1-FRCM3 load/unload	850.4	1843.9	0.56	1.48	0.20	0.53	0.062	0.16
Average	785.6	1739.4	0.52	1.42	0.18	0.39	0.072	0.15

Table 7
Tensile tests results on G-FRCM.

	E_1 (GPa)	E_2 (GPa)	E_3 (GPa)	σ_{t1} (MPa)	σ_{t2} (MPa)	σ_u (MPa)	ε_{t1} (%)	ε_{t2} (%)	ε_u (%)	E_1^* (GPa)	σ_{t1}^* (MPa)
Average	1310.16	32.33	64.22	460.05	431.11	872.17	0.045	0.38	0.69	5.89	2.07
Min value	980.40	20.04	38.32	288.88	340.23	621.32	0.035	0.34	0.42	4.41	1.29
Max value	2161.51	41.58	83.95	644.45	550.14	1127.78	0.091	0.46	1.10	9.73	2.90
CoV [%]	33.49	34.38	17.03	29.90	19.59	21.29	40.71	12.61	37.95	33.50	29.90

of the monotonic tests. The three tests showed a percent reduction of the maximum load with respect to the average failure load of the monotonic tests respectively equal to 43.6%, 49.7% and 36.6%.

The initial part of the reloading path, at least in the first cycles, is apparently similar to the elastic stiffness of the starting cycle. For higher loads the slope of the reloading path decreases. This aspect is reduced in the following cycles. The residual dimensions of the cracks after the un-loading cycle were measured at the second transition point (T_2) and at collapse analyzing the digital images and the residual displacements and strains recorded during the tests. Table 6 shows these values.

5.2. G-FRCM

Tensile tests on G-FRCM show mainly a bi-linear behavior since only in few tests a third phase is recognizable and in most of the tests the second and third phases are comparable (Fig. 12).

Table 7 shows the average tensile results of 17 tests on G-FRCM, the values reported were calculated as explained in the previous paragraph. The stress was calculated dividing the loads by the section of the fabric (Fig. 12). Even in this case the last two columns in Table 7 report the first elastic modulus and the stress corresponding to T_1 calculated with reference to the mortar area.

The stiffness of the third phase (64.22 GPa) could be compared with the elastic modulus of the glass fiber grid (55.6 GPa).

5.3. C-FRCM

Tensile tests on C-FRCM show a tri-linear behavior like tests with PBO-FRCM. A phenomenon of slippage was noted in many tests (Fig. 13) due to problems of bond and impregnation between the mortar and the dry carbon fiber. Damage of carbon textile was observed close to the main cracks.

10 tests were carried out on C-FRCM and the stress-strain curves are reported in Fig. 13. Table 8 shows the tensile test results. The modulus of the third phase (186.42 GPa) was slightly lower than the one of the dry carbon textile (203 GPa) because of the phenomena of slippage between carbon fibers and mortar.

The load-displacement curves in Fig. 13 highlight the slippage phenomena that are not evident in the stress-strain plots. The slippage is represented in the decreasing last branch of some curves. This is not evident in Fig. 14 because the stress-strain curves are recorded by the extensometer that was removed just before the ultimate failure. This branch can indicate a fourth zone of the load-displacement curves as reported in [22]. This phenomenon was interpreted in [19] as a consequence of measuring the average strains along the entire height of the specimens and therefore including the pull-out effects.

Even in this case two tensile cyclic tests were carried out. The test set-up was the same of monotonic tests and the cycles were done at steps of 0.2 kN. Fig. 15 shows a stress-strain curve and a comparison with a corresponding monotonic curve showing a good

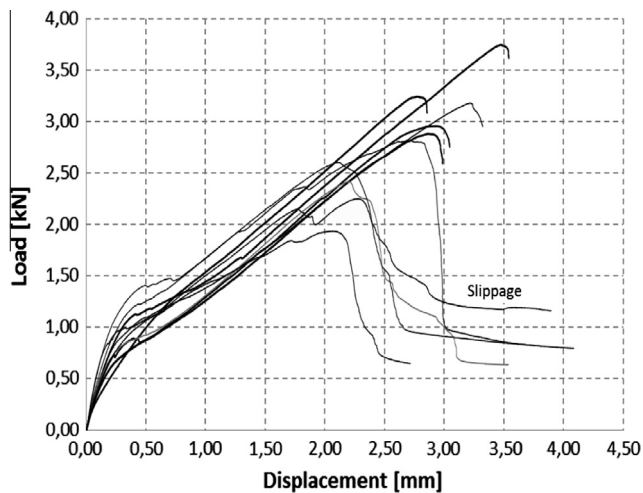


Fig. 13. Load-displacement curves for C-FRCM specimens.

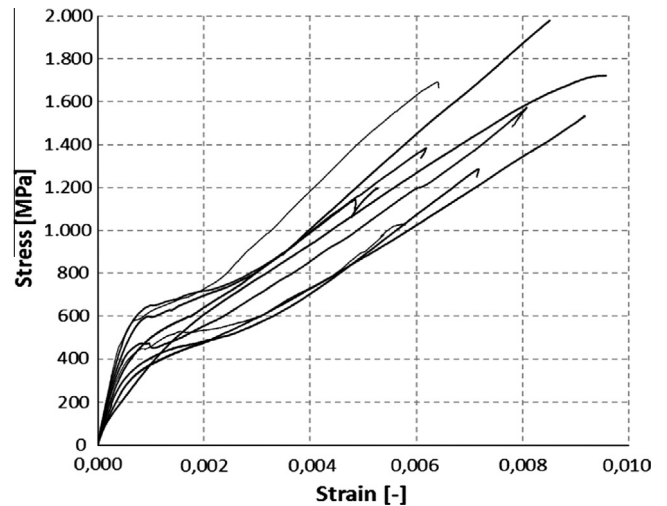


Fig. 14. Stress-strain curves for C-FRCM specimens.

Table 8
Tensile tests results on C-FRCM.

	E_1 (GPa)	E_2 (GPa)	E_3 (GPa)	σ_{t1} (MPa)	σ_{t2} (MPa)	σ_u (MPa)	ε_{t1} (%)	ε_{t2} (%)	ε_u (%)	E_1^* (GPa)	σ_{t1}^* (MPa)
Average	1102.2	68.23	186.42	481.72	619.56	1492.02	0.06	0.24	0.74	4.96	2.02
Min value	802.4	42.44	136.10	332.07	480.00	1031.91	0.05	0.17	0.52	3.37	1.39
Max value	1519.1	95.02	245.00	614.74	748.90	1994.68	0.07	0.31	0.99	6.38	2.58
CoV [%]	17.5	28.28	22.17	21.42	19.12	18.82	12.76	20.04	21.01	17.59	21.42

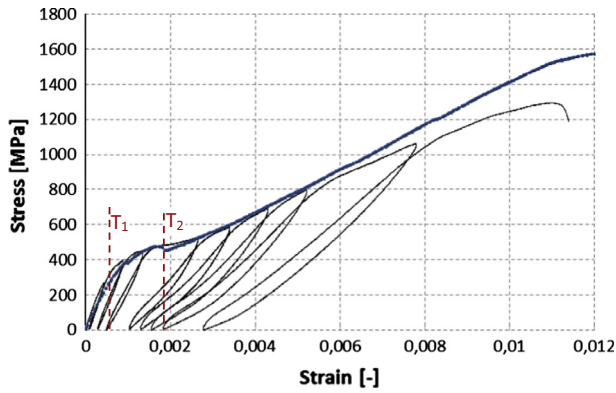


Fig. 15. Stress–strain curves, comparison between monotonic test and load-unload cycling test.

correlation. At the second transition point (T_2) and at collapse the residual dimensions of the cracks after the un-load cycle were measured analyzing the digital images and the residual displacements and strains recorded during the tests. Table 9 shows these values. The two tests showed a percent reduction of the maximum load respect to the average failure load of the monotonic tests equal to 12.8% and 7.8% respectively.

Fig. 16 shows a comparison of tensile tests results on PBO-FRCM, G-FRCM and C-FRCM materials. The stiffness of the first and second phases of the three materials are comparable. Instead there are many differences in the third phase, due to the different elastic moduli and tensile strengths of the fibers. The ultimate strength of PBO-FRCM is the highest but the envelope is significantly large. On the contrary the small variability of the results for G-FRCM is correlated to the lowest mechanical properties.

Table 10 reports a comparison between the maximum stresses and strains reached in tensile tests on FRCM materials and on the dry fibers and the strains reached at the end of each phase with respect to the maximum strain. The results highlight a good correlation between the strength of FRCM materials and its components. This indicates that the stresses are completely transmitted from the matrix to the textile. The results of C-FRCM are lower because of the slippage phenomena developed between matrix and dry fibers. The ratio between the strains at the end of the first and second phases show that the second phase is characterized by the properties of each material but this value increases for greater distances between the rovings in the weft direction in the textile. Finally, the ratio of the average strains at the end of the second phase and at failure are values close to 3.3. This consideration, even if limited to the present investigation, could be included in possible guidelines for FRCM.

6. Crack spacing measurements

The crack development throughout the loading cycle of the tensile tests was recorded by digital images of the specimens at regular load-deformation increment intervals. The trends of the crack

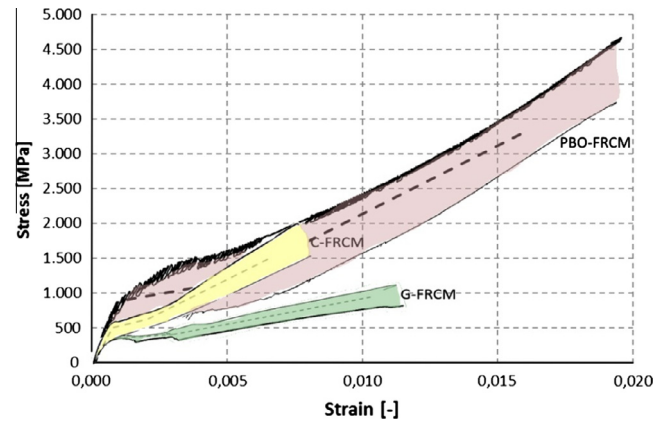


Fig. 16. Stress–strain curves: PBO-FRCM, C-FRCM, G-FRCM with average values (dashed lines).

spacing are analyzed in relation to the stress level reached. The crack spacing was reduced in the first part of the test and then it reached a level that is called the “saturation crack spacing” [22]. After this point, there was an increase of the size of the cracks, without formation of further cracks. For PBO-FRCM samples (Fig. 17) the saturation crack spacing was variable from 19 mm to 25 mm while for G-FRCM (Fig. 18) was approximately of 50 mm and for C-FRCM (Fig. 19) it was variable from 26 mm to 30 mm. In order to better analyze the cracking phase and facilitate the comparisons, in these figures the stresses are calculated with respect to the cross section area of the mortar for each specific specimen. An anomalous point is visible in Fig. 17. One of the specimens for a stress between 2.0 and 2.5 MPa presented an increase of the crack spacing during loading. This is due to the fact that in the first phase the location of the cracks is not homogeneous on the specimen. In this particular case the crack spacing between the first and second cracks was equal to 55 mm. Afterward the third crack appeared at a distance of 85 mm from the first. So the average crack spacing at this stress level became equal to 70 mm.

Fig. 20 shows the trend of the crack size with respect to the reached deformation level. A typical curve is shown for each material. In PBO-FRCM specimens, when the level of saturation crack spacing was reached (with a deformation equal to 0.01), the crack increase up to a maximum value before the collapse which varies between 0.29 mm and 0.50 mm. In C-FRCM a similar behavior could be highlighted, and the size of the cracks before the collapse varies between 0.30 mm and 0.45 mm. In G-FRCM the behavior was similar, but the level of saturation crack spacing is reached for a deformation of 0.0015. The maximum cracks size was variable from 0.63 mm to 0.92 mm.

These results show how the different size of the fiber grids and the geometries influenced the cracks propagation and their maximum dimensions. The PBO grid had a dimension of 20 mm in the weft direction and this caused a saturation crack spacing of about 22 mm. On the contrary this cannot be the explanation both in the case of C-FRCM specimens where the grid is 10 mm against a

Table 9
Load-unload tests results on C-FRCM.

Sample	Stress (MPa)		Residual displacement (mm)		Residual strain (%)		Average crack residual dimension (mm)	
	T_2	Collapse	T_2	Collapse	T_2	Collapse	T_2	Collapse
C-FRCM1 load/unload	759.2	1586.8	0.21	0.49	0.087	0.21	0.052	0.096
C-FRCM2 load/unload	570.3	1048.3	0.35	0.67	0.147	0.29	0.088	0.116
Average	664.7	1317.5	0.28	0.58	0.117	0.26	0.070	0.063

Table 10
Comparison between stresses and strains reached in different phase of the tensile tests.

Materials	$\sigma_{\text{FRCM}}/\sigma_{\text{dry fibers}}$	$\varepsilon_{\text{FRCM}}/\varepsilon_{\text{dry fibers}}$	$\varepsilon_{\text{FRCM phase2}}/\varepsilon_{\text{FRCM phase1}}$	$\varepsilon_{\text{FRCM max}}/\varepsilon_{\text{FRCM phase2}}$
PBO-1	0.97	0.85	6.25	3.38
G	0.89	0.81	8.33	3.48
C	0.78	0.82	3.83	3.22

saturation crack spacing of 28 mm and in the case of G-FRCM specimens with a grid of 15 mm against a saturation crack spacing of approximately 50 mm. Further investigation is needed to clarify this aspects connected to the grid geometry. For example, techniques as thermography could be adopted to study the cracks location with respect to the transversal yarns geometry.

7. Mechanical properties of FRCM systems applied to a brick

The behavior of the FRCM system applied on a brick substrate was investigated to analyze both the adherence between the reinforced system and the substrate and between the dry textile and the cementitious mortar. The experimental campaign was developed on different FRCM materials: PBO2-FRCM, G-FRCM, C-FRCM and PBO-G-FRCM.

Two different types of bricks were used: the PBO2-FRCM, PBO-G-FRCM and C-FRCM were applied on bricks with a compressive strength of 20.8 MPa [33] while the G-FRCM was applied on bricks with a compressive strength of 68.8 MPa [27]. The pull-off resistance of this single brick used with G-FRCM was equal to 3.1 MPa. The large difference in the compressive strength of the bricks was motivated by the need to validate the tests with very different masonry substrates.

7.1. Push-pull double lap tests

Many methods have been developed to analyze the bond between reinforced materials (FRP and FRCM) applied on concrete or masonry substrates. The experimental set-ups include single

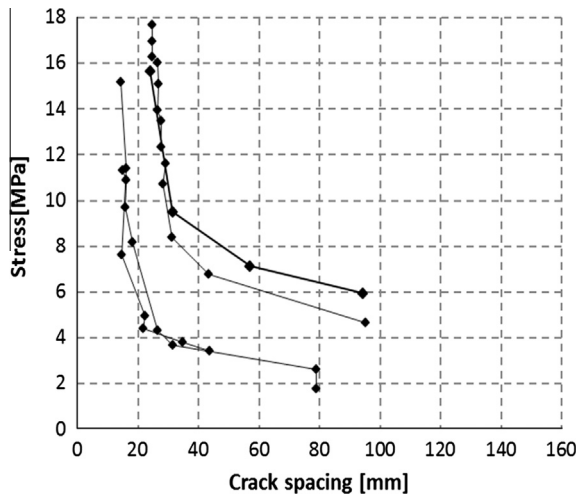


Fig. 17. Crack spacing vs stress. PBO-FRCM specimens.

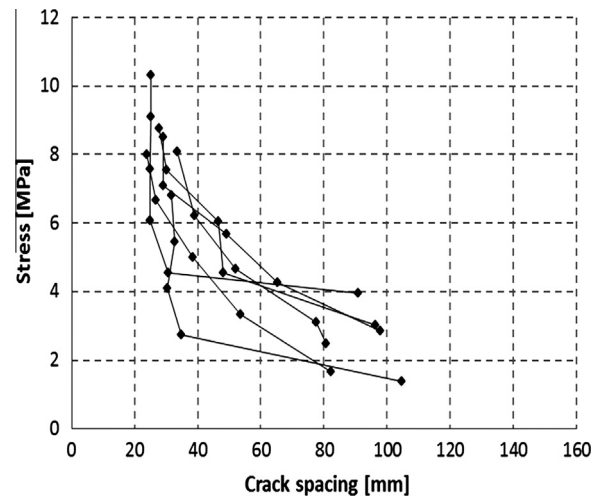


Fig. 19. Crack spacing vs stress. C-FRCM specimens.

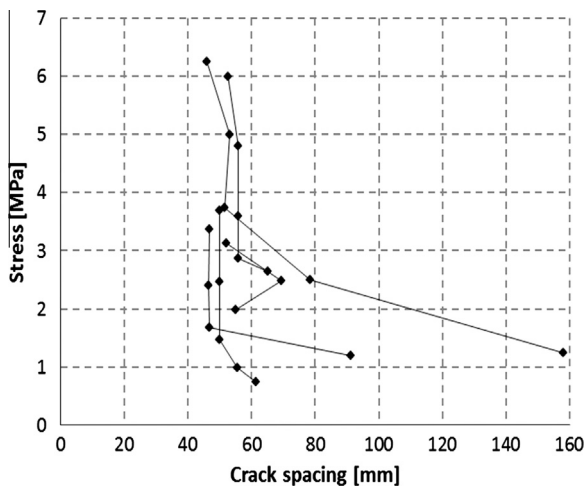


Fig. 18. Crack spacing vs stress. G-FRCM specimens.

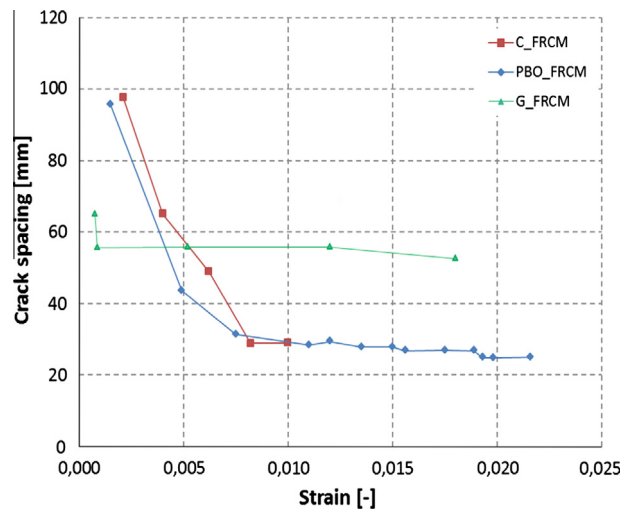


Fig. 20. Comparison of Carbon, PBO1 and Glass FRCM. Crack size vs strain.

[23,24,36] or double laps [4,24–26,37] push–pull shear tests. In order to avoid eccentricity phenomena and to verify the efficiency of the methodology, a double lap set-up was preferred in this work. A recent wide experimental campaign on masonry, detailed in [24], demonstrated that this testing set up is easy, economic and reliable.

An even distribution of the load between the two laps may be usually assumed. This could be verified in each test applying two instruments on the opposite sides of the bricks but with extra costs. During all the tests great attention was paid to verify the symmetry and the similar behavior of the two laps during the loading.

The testing rig for double-lap experiments was made with a steel frame composed of two transversal beams connected by four bars (Fig. 21). In order to minimize the effects of possible misalignments a spherical joint was positioned in the upper part of the rig. To guarantee even distribution of the load on the composite strips, the textile was placed around a cylinder with a diameter equal to the thickness of the brick and the relevant mortar. Two Teflon sheets were used to avoid friction and facilitate the even distribution of the load.

Two different geometries of the reinforcements were considered: glass fiber grids were bonded to two opposite sides of a single brick [27]. On the contrary carbon, PBO and PBO-G grids were bonded on a specimen composed of three bricks and two mortar joints.

Different bond lengths and widths were considered in order to investigate the effect of these parameters on the debonding strength and failure modes. Table 11 shows the layout of the experimental tests.

A special test frame was designed and realized to perform the tests (Fig. 21).

7.2. Experimental results

The results of the tested series of specimens are here reported in terms of load–displacement curves. In particular the total applied load is considered while the relative displacement of the grips of the testing machine was recorded deducing the elongation of the dry strip in the unbounded textile (Fig. 21). The stiffness of the setup was verified and its deformability was evaluated negligible.

• G-FRCM

As expected, the bond length had an influence both on the failure load and on the failure mechanism. The failure load increased with the bond length. Short reinforcements (50 and 100 mm) showed grid slippage at loads smaller than the tensile failure of the glass fibers (Fig. 26a). On the contrary, for longer bond lengths a tensile failure of the glass fibers grid was experienced. No debonding of the FRCM system from the substrate was detected in all specimens. Among the tested lengths 150 mm provides the best results, with an average failure load equal to 4.06 kN (Table 12).

In Fig. 22 a comparison of the experimental load–displacement curves obtained for different bond lengths is represented. The differences in the failure modes and in the maximum loads are reported in Table 12 for the G-FRCM specimens. The average ultimate values vary from 1.0 kN for a bond length of 50 mm to 4.1 kN for a bond length of 150 mm.

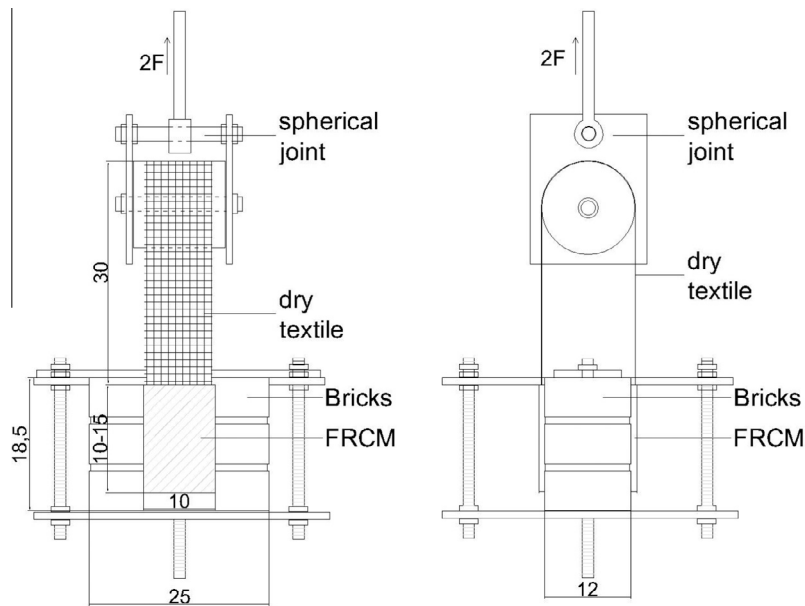


Fig. 21. Push–pull double lap tests set up (the measures are expressed in cm).

Table 11
Geometry of double lap tests.

Reinforcement type	Bond length (mm)			Reinforcement width (mm)	Substrate
G-FRCM	50 (5)	100 (5)	150 (5)	50 (3 yarns)	Single brick
PBO2-FRCM	–	100 (3)	150 (4)	100 (7 rovings)	Three bricks
C-FRCM	–	100 (3)	150 (4)	100 (11 rovings)	Three bricks
PBO-G-FRCM	–	100 (3)	150 (4)	100 (7 rovings)	Three bricks

Note: Within brackets # of samples.

Table 12
Main experimental results of push-pull tests.

Reinf. type	Bond length (mm)	Initial stiffness (N/mm)		Failure load (kN) ^a		Stress in yarns at failure (MPa) ^b	Textile stress/textile strength (%)	Failure mode
		Average	Standard deviation	Average	Standard deviation			
G-FRCM	50	697	63.7	1.06	0.17	196.2	16.3	Slippage of the grid
	100	644	17.2	2.63	0.17	487.0	58.4	Slippage and subsequent partial failure of the grid
	150	670	64.4	4.06	0.55	751.8	62.6	Tensile failure of the grid
PBO2-FRCM	100	1042	25.1	7.83	0.69	2542.2	80.0	Partial fiber failure in weft and warp directions
	150	739	155.4	7.56	1.07	2454.5	77.3	
C-FRCM	100	1703	152.9	10.19	0.31	1102.8	57.6	Partial fiber failure in weft and warp directions and subsequent slippage of the grid
	150	1504	289.2	10.42	0.55	1127.7	59.0	
PBO-G-FRCM	100	680	71.5	4.26	0.40	1776.6	58.9	Complete tensile failure of the glass fibers and partial failure of the PBO fibers
	150	701	37.7	4.64	0.76	1933.3	66.3	

^a Total load applied to the specimen.

^b Theoretical stress assuming a uniform distribution in the yarns.

- PBO2-FRCM

The tests on PBO2-FRCM samples showed a small influence of the bond length (100 and 150 mm) both on the failure mechanism and load. The collapse was not abrupt with sudden breaking of all the fibers but it was characterized by a progressive failure of the rovings.

First of all the rovings in the weft direction collapsed, after that also the rovings in the warp direction gradually failed (Fig. 26b). The rovings in the external part of the grid are subjected to greater stresses. No slippage was observed between the grid and the cementitious matrix. In Fig. 23 a comparison of the experimental load-displacement curves obtained for different bond lengths is represented. The main experimental results are reported in Table 12.

- C-FRCM

Even the tests on C-FRCM samples showed a small influence of the bond length (100 and 150 mm) both on failure mechanism and load. The failure mode presented a slippage located between the grid and the cementitious mortar after the a partial failure of the fibers in weft and warp directions (Fig. 26c). The collapse was not abrupt including all the fibers but it was characterized by a progressive failure of the rovings similar to the failure mode analyzed for PBO2-FRCM.

In Fig. 24 a comparison of the experimental load-displacement curves obtained at different bond lengths is represented. In Table 12 the main experimental results are reported. The slippage phenomena are difficult to identify in the load-displacement curves because the fiber-matrix slip occurs during the collapse of the system, after the partial fiber failure in weft and warp directions.

- PBO-G-FRCM

The tests on PBO-G-FRCM samples do not show an influence of the bond length (100 and 150 mm) on failure mechanism or failure load. The failure mode showed first of all the collapse of the glass fibers at relatively low load and after that a gradual failure of the PBO fibers in the weft and warp directions was experienced (Fig. 26d). No slippage was observed between grid and cementitious mortar.

In Fig. 25 a comparison of the experimental load-displacement curves obtained for different bond lengths is represented. In Table 12 the main experimental results are reported.

The experimental results showed two main failure modes: in C-FRCM specimens a failure in the fiber-matrix interface and the slippage of the grid in the mortar layer, in PBO-FRCM a partial failure of the fibers in weft and warp directions without slip between fibers and mortar. The tests carried out on G-FRCM showed the importance of the bond length in the occurrence of slippage

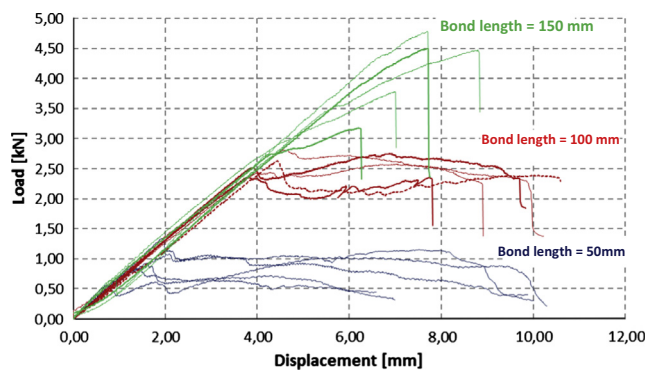


Fig. 22. Load-displacement curves for G-FRCM samples.

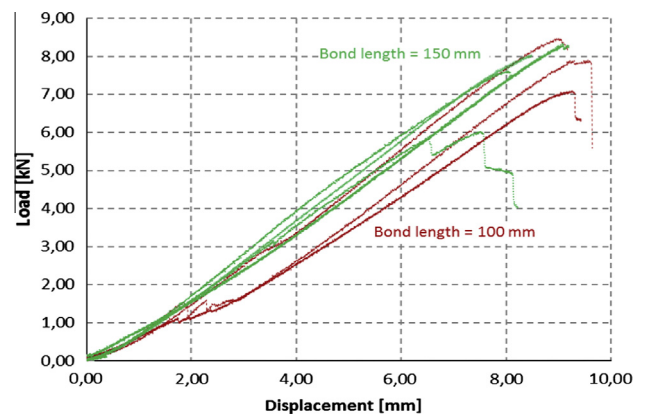


Fig. 23. Load-displacement curves for PBO2-FRCM samples.

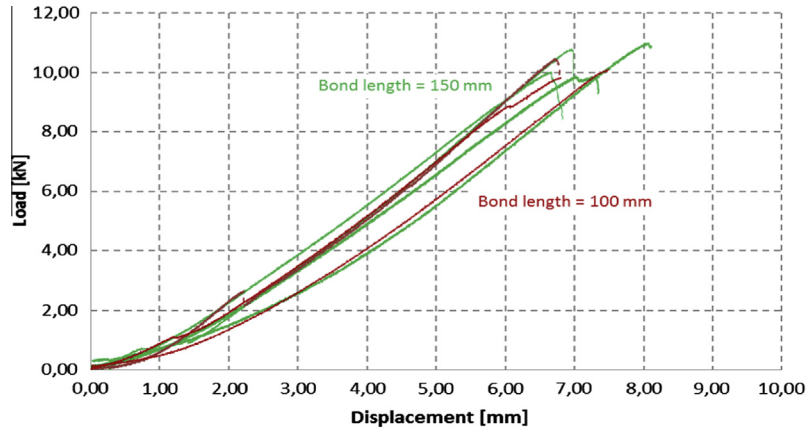


Fig. 24. Load–displacement curves for C-FRCM samples.

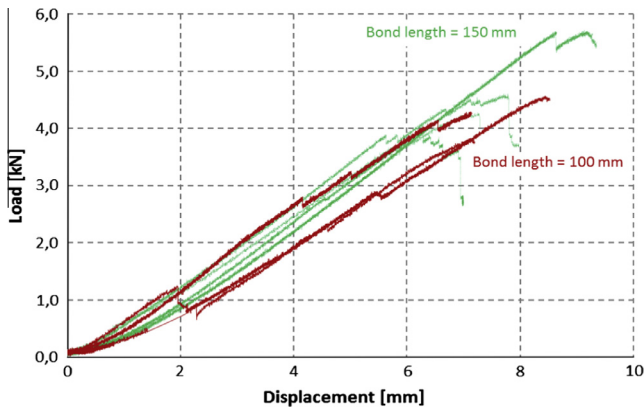


Fig. 25. Load–displacement curves – PBO-G-FRCM samples.

phenomena. For bond length equal to 150 mm the textile failure occurred, in the other cases slippage phenomena were detected.

In Table 12 the theoretical ultimate stress of the fibers assuming a uniform distribution in the yarn is also reported together with the ratio of the stress in the yarns with the tensile strength. These were calculated assuming a uniform distribution of the stresses in the yarns. This is an evident approximation but permits to estimate the ratio of the stress at failure against the ultimate strength of the fibers. All the failures detected in the experiments included only

part of the fibers in the yarns for the so-called telescopic effects. Another factor that should be considered is the different area of influence of the rovings closer to the longitudinal edges. The width of the matrix layer is larger than the width of the textile of about 20 mm per side, so the area of influence of the external rovings is greater with respect to the internal ones.

It is known that a cement matrix is not ideal to impregnate filaments. The external filaments in a roving are in direct contact with the matrix and tightly bonded, while the internal filaments in the core of the yarn are not in direct contact with matrix and can slip more easily because of the low friction between the fibers.

Peled et al. [28], Soranakom and Mobasher [29] and Andic-Cakir et al. [30] presented a similar model to simulate the roving as a cylindrical structure comprised of concentric rings, each one composed of several filaments (Fig. 27). The failure mode of the filaments in the sleeve is the fracture while the internal filaments present a slippage due to the pull-out force.

This telescopic mode of pull-out [31] is influenced by the cement penetrability, the geometry of the reinforcement, the presence of a coating and the level of friction between the fibers in each yarn. The last can vary with the different qualities of the fibers. In particular it is believed that it is significant in the case of carbon fibers and that the PBO-FRCM specimens present a better bonding and a better stress transfer between matrix and filaments than the carbon or glass fibers. The PBO textile presents a geometry that facilitates the penetration of the mortar in the grid and an high friction level between the filaments due to the physical and chemical properties of the material.

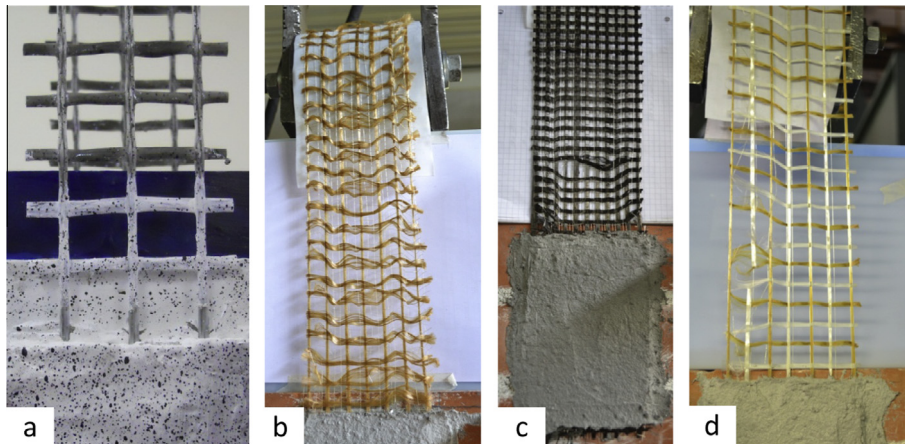


Fig. 26. Failure mode of push–pull double lap tests: (a) G-FRCM sample (bond length equal to 50 mm); (b) PBO2-FRCM; (c) C-FRCM; (d) PBO-G-FRCM.

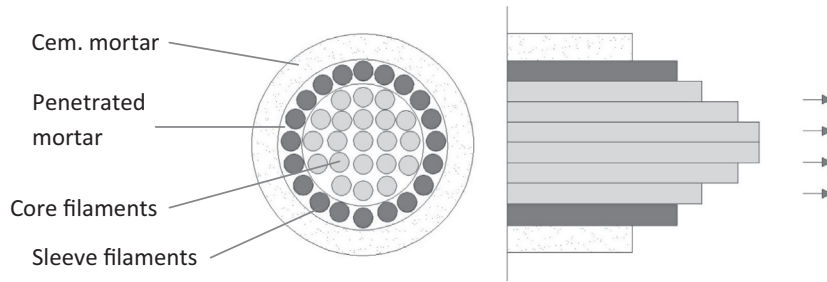


Fig. 27. Telescopic behavior: idealization of a roving embedded in the mortar [31].



Fig. 28. Failure mode of pull-off tests: (a) G-FRCM sample mode (c); (b) G-FRCM sample mode (b); (c) C-FRCM mode (a).

Table 13
Average results of pull-off tests.

	Reinforcement type	# Tests	Maximum load (kN)	Maximum stress (MPa)	Failure mode
(1)	G-FRCM	2	2.9	1.48	(c) Interface mortar-textile
		3	2.6	1.34	(b) Interface substrate-composite
(2)	PBO2-FRCM	6	2.1	1.08	(a) In the substrate
(3)	C-FRCM	6	2.2	1.14	(a) In the substrate
(4)	PBO-G-FRCM	6	2.0	1.03	(a) In the substrate

7.3. Pull-off tests

Pull-off tests are significant to determine the tensile strength of the FRCM composite after the material has been applied to a surface. The tests were performed on specimens of FRCM bonded to the surface of a clay single brick according to EN 1542 [32]. The mechanical characteristics of the bricks are reported in the first part of Chapter 7. The test specimen is formed by drilling a shallow core perpendicular to the surface and leaving the core attached to the brick. This had a depth of 10 mm. A steel disk was attached to the external surface of the core using epoxy adhesive. A tensile loading device was applied to the test specimen coincident with the axis of the specimen. The steel disk had a nominal diameter of 50 mm and a thickness of 25 mm.

The failure mode can occur:

- (a) In the substrate.
- (b) At the interface between the substrate and the composite.
- (c) At the interface between the mortar and the textile.
- (d) In the mortar without involving the textile.
- (e) At the interface between the composite and the epoxy adhesive used to fix the steel disk.

Four groups of specimens were tested and the results are reported in Table 13. The G-FRCM specimens showed two different failure modes. Two of them collapsed at the interface between the mortar and the substrate (Fig. 28b). In the others the failure was localized at the interface between mortar and textile (Fig. 28a).

It should be reminded that two different types of bricks were used as reported in Chapter 7.

The obtained results of these pull-off test reflected the different resistance of the bricks. Specimens of series (1), made with G-FRCM, were applied on the bricks with a compressive strength of 68.8 MPa and a pull-off resistance of 3.1 MPa. The resulting pull-off load varied between 1.34 and 1.48 MPa. The remaining series of specimens were bonded on bricks with lower mechanical properties and showed an ultimate value coincident with the ultimate resistance of the bricks (approximately 1 MPa).

8. Conclusions and considerations for future guidelines

A mechanical characterization of FRCM materials was carried out on different series of specimens. Five types of FRCM materials were studied, composed by different types of PBO, carbon and glass fibers and three types of mortars.

Most of the experiments confirmed that the behavior of FRCM materials in tension can be characterized by trilinear curves. The first part corresponds to the un-cracked phase of the mortar while in the second phase the cracks develop and in the third phase only the fiber reinforcement can carry the applied load. In this phase the measured stiffness and the ultimate strength of the specimens correspond to the relevant values of the dry fabrics.

From the tensile test results on PBO-FRCM, G-FRCM and C-FRCM materials it is possible to conclude that the slopes of the first and second phases are comparable. Instead there are many differences in the third phase, due to the different modulus and

tensile strengths of the fibers. It is evident that the ultimate strength of PBO is the highest but the envelope of the results is significantly large. On the contrary the small variability of the results for G-FRCM is correlated to the lowest mechanical properties.

The fabric with the highest performance (PBO) provided the best composite behavior in terms of ultimate strength even if the scatter of the results was larger. The specimens made with PBO fibers showed also the best bond properties. The push-pull double lap tests and the pull-off tests showed a good adhesion between the masonry substrate and the FRCM system. Slippage phenomena can occur in samples made with carbon fibers in the third phase of the tensile tests or before the collapse in push-pull shear tests.

The authors believe that the experimental set-up and the results proposed in this paper could be of help to define guidelines for the mechanical characterization of FRCM materials. In particular the following aspect should be considered. The mechanical properties of the composites are influenced by the presence of small defects in the cementitious matrix, by the non-constant dimensions of the section and by the localization of the first cracks with respect to the extensometer. The literature review showed different experimental set-ups used for tensile tests on FRCM materials. The set-up here proposed is the most efficient to avoid damage of the samples and slippage in the grips.

The guidelines shall indicate the method to identify the points of transition between the three phases. If these points are not clearly detected, it is possible to draw the tangent lines and find their intersection. All these parameters must be calculated using the area of the textile. In the un-cracked first phase, the elastic modulus and the maximum stress could be calculated also using the minimum area of the matrix, after a proper detailed geometrical measurement of the thickness and width of the specimens, in order to compare these values with the mechanical properties of the matrix. The analysis of the cracks spacing and size showed a good correlation between the geometry of the fabric and the location of the cracks. This could be an important parameter in the design procedures.

It is suggested that the push-pull and the pull-off tests should be proposed in the guidelines as tools to evaluate the adhesion of the FRCM to the masonry substrate. Among different available test set-ups for push-pull tests, the authors believe that the double lap, as presented in this paper, is the more convenient both in terms of costs and to avoid alignment problems.

A problem that was not considered in this paper refers to the dynamic performance of the FRCM reinforcement [39]. This important issue could be investigated in future steps of this research project.

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