

1 **OPTIMIZATION OF PUMPING STATIONS IN COMPLEX WATER SUPPLY NETWORKS**
2 **THROUGH EVOLUTIONARY COMPUTATION METHODS.**

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11 **Abstract**

12 The proper scheduling of pumps operations in water supply systems yields to energy cost-
13 savings. The pumps schedule is the set of many combinations of pumps operation parameters,
14 variables in time, which must fulfil the system restrictions. Traditional approach to this problem
15 leans on the man's experience, while in the last decades many optimization procedures have
16 been developed.

17 In the paper, a method based on Genetic Algorithms to optimize the pumps functioning in water
18 distribution networks is chosen and described. The purpose of the optimization is to reduce the
19 energy consumption while maintaining a good service, considering the problems related to the
20 calibration of a high number of parameters and many constraints.

21 The developed method is then applied to the water supply network of Milano, Italy, which is a
22 large and complex system with no suspended reservoirs because the hydraulic head is
23 maintained by the action of 31 pumping stations. On the basis of real data, and following a field
24 survey, the operations of the entire network and its pumping stations, with the actual scheduling,
25 are firstly simulated with a purpose-developed software which uses the Epanet Toolkit, and then
26 optimized with the proposed method.

27 The results of the applied optimization procedures show that the new operational system
28 produces a significant improvement and economic saving.

29 *Keywords:* water distribution, genetic algorithms, optimization, energy saving.

30 **1. Introduction**

31 Water distribution networks are crucial components of water supply systems. They are among the
32 main infrastructure assets of a society, and their management must be effective, efficient and energy
33 saving. The hydraulic head of large towns is very often completely maintained by the action of a
34 number of pumping stations: in such systems a good management of the pumping station, in order to
35 minimize the power consumption, is a critical aspect. To this end, hydraulic constraints are not
36 sufficient to find the best system, which requires that economic and energy conditions be taken into
37 account. In order to handle this kind of complex and multi-objective optimization problem
38 Evolutionary Computation (EC) methods are often used. Because of the large number of parameters
39 and the complexity of the system, the early methods based on linear or non-linear programming or,
40 even worse, complete enumeration, are not applicable. Starting in the early Nineties, the different
41 existing (meta)heuristics frameworks have been applied to solve the water supply networks related
42 optimization problem. The taxonomy reported in De Corte and Sörensen (2012) divides metaheuristics
43 frameworks into three classes: (i) *Local search metaheuristics*, which operate on a single complete
44 solution and iteratively improve it by making small adjustments called moves; (ii) *Population-based*
45 *metaheuristics* operate on a set of solutions and find better solutions by combining solutions from that
46 set into new ones; (iii) *Constructive metaheuristics* build a solution by working with a single,
47 unfinished, solution and adding one solution element at a time.

48 Genetic Algorithms (GA) (Goldberg 1989) methods, which belong to the “population based
49 metaheuristics”, are often chosen. GA are a technique of Evolutionary Computation appropriate for
50 combinatorial optimization (Eiben and Smith, 2003) and their use is central in modern water resources
51 planning and management. They have been widely used in the past two decades to improve the
52 efficiency of water distribution systems when traditional calculus-based and enumerative optimization
53 methods have proved unable to deal with the geometrical complexity of those systems. One of the first

54 water engineering applications of GA was related to the optimization of pump schedules for a serial
55 pipeline (Golberg and Kuo, 1987; Simpson et al, 1994). GA have been also used for least-cost design
56 (Savic and Walters, 1977); as mentioned, network optimization has also been studied, applying
57 different methods (Schaake and Lai, 1969; Alperovits and Shamir, 1977; Fujiwara and Khang, 1990).
58 Mackle et al. (1996) were among the first to apply a binary GA to pump scheduling problems in order
59 to minimize energy costs, subject to reservoir filling and emptying constraints. Savic and Walters
60 (1997) developed a multi-objective GA (MOGA) approach to minimize both the energy costs and the
61 number of pump switches, while Wu et al. (2009) directly computed the pump speed through the
62 matrix system of a global gradient algorithm. The first industrial applications of GA to the pump
63 scheduling problem are reported in (De Schaetzen et al., 1998; Illich and Simovic, 1998; Atkinson et
64 al., 2000). Van Zyl et al. (2004) developed a hybrid optimization approach in order to reduce
65 excessive running times. Further computational efficiency have been achieved by Rao and Salomons
66 (2007) through their new approach for the real-time, near-optimal control of water-distribution
67 networks through a process based on the combined use of an artificial neural network for predicting
68 the consequences of different control settings and a genetic algorithm for selecting the best
69 combination. Prasad and Park (2004) developed a multiobjective GA approach to the design of a water
70 distribution network, the objectives considered were minimization of the network cost and
71 maximization of a reliability measure. Shamir and Salomons (2008) used a reduced model of the
72 network which reproduces its performance over time with high fidelity with optimization by a genetic
73 algorithm. Costa et al. (2010) used GA for optimizing the water supply system operations in the city of
74 Ourém (Portugal). Behandish and Wu (2013) applied a modified GA able to deal with integer
75 parameters and ANN to predict the hydraulic state of water distribution.

76 Dealing with real networks is not a simple task, and this is probably the reason why Walski et al.
77 (2010) stated “A great deal of research has been conducted on optimizing pumping operation but such
78 optimization is still not widely used.” therefore a number of different solutions has been developed.

79 In this paper, GA have been selected as the Authors found (Becciu et al., 2014) that methods
80 belonging to the “Local search metaheuristics” risk to be trapped in unfeasible spaces, while methods
81 belonging to “Constructive metaheuristics” are promising but quite difficult to be calibrated and

82 requires further research. Therefore, a simple GA is set out in order to improve the performance of the
83 water distribution system in Milano (Italy) in terms of power consumption at pumping stations, trying
84 to assess whether this method is appropriate to be used in the field. In Milano, as mentioned, similarly
85 to most of large cities, there are no reservoirs and the hydraulic head is given by 26 pumping stations
86 positioned quite uniformly all over the city.

87 Results show that significant improvements can be achieved in terms of savings of energy and,
88 therefore, of money. Tests on a pumping station demonstrate that the results obtained are reliable.
89 Finally, GA are metaheuristic methods that can be successfully applied to pumps optimization in
90 complex systems; they have been shown to be stable and to bring to good results. Possible
91 improvements are investigated and briefly described in the conclusions.

92 **2. Optimization algorithm**

93 The aim of the research was to find the best scheduling of pumps functioning, able to ensure the same
94 level of service now guaranteed with the minimum energy expenditure. Water is presently distributed
95 assuring that heads in all the nodes of the network are higher than 25 m. This is an optimization
96 problem with constraints, and Genetic Algorithms technique is found to be appropriate among the
97 Evolutionary Computing “dialects” (Eiben and Smith, 2003; Becciu et al., 2015).

98 The method is implemented in a purpose developed software, which uses the Epanet Toolkit to solve
99 the network once a new scenario is defined.

100 The status of the pumps has been implemented in a binary string, where each pump is represented by 3
101 bits (i.e. $2^3 = 8$ possibilities) which describe its working status, i.e.: pump off, pump on working at 40,
102 50, 60, 70, 80, 90 and 100% of its nominal speed.

103 The objective function (O.F.) to be minimized is the power required by the system W (and
104 consequently the required energy) which is obtained by summing the power required by each running
105 pump:

$$106 \quad O.F. = W = \sum_{i=1}^{N_{\text{pump}}} \frac{\gamma H Q}{\eta} \quad (1)$$

107 where γ is the specific weight of the fluid (water), H is the heads of the pumps for the discharge Q ,
108 and η is the efficiency of the pump; this O.F. is minimized every full hour.

109 The first step in the implementation of the GA is the generation of a number of configurations that are
110 used as the first generation of solutions (“population”): one of these is the actual configuration, the
111 others are created randomly by the software.

112 New populations are generated starting from the previous with the usual methods of cross-over and
113 mutation (Goldberg, 1989; Eiben and Smith, 2003). Moreover, a 10% of elitism has been used in order
114 not to lose the best individuals.

115 Each configuration (individual) is simulated using the Epanet Toolkit in order to evaluate the
116 correspondent value of the objective function (i.e. its fitness), and to check the constrains. If the
117 pressure at some control points falls below the above-mentioned value of 25 m, the value of the *O.F.*
118 is increased in function of the difference from the acceptable (*MinPres*) and the computed (*CompPres*)
119 pressures in order to penalize the configuration; the O.F. therefore becomes:

$$120 \quad O.F. = W \cdot 5 \cdot \left(1 + \frac{MinPres - CompPres}{MinPres}\right) \quad \text{when } CompPres < MinPres \quad (2)$$

121 where *W* is defined with the eq. (1) and, as mentioned, *MinPres* = 25 m. Obviously, the penalty
122 functions are not derived from theoretical consideration, but depend on the problem to be studied; in
123 this case the function is studied in order to increase the required power of 20% per meter; moreover, a
124 check on the respect of the condition $MinPres \geq 25 \text{ m}$ has been performed on all the carried out
125 solutions, and it has been found that the “best” solutions satisfy this requirement.

126 Optimization is run hour by hour, so the daily optimization is given by the combination of hourly best
127 results. Figure 1 shows the framework of the whole procedure. Because of the intrinsic randomness of
128 the algorithm, each simulation may produce a different result; therefore, for every hour of the day 10
129 simulations were run with different starting points, i.e. different initial populations, composed by one
130 individual equal to the actual configuration and the others randomly generated.

131 **3. Case study: the Milano water supply network**

132 The water supply system of Milano (Motta 1989) acquires drinking water from a number of wells, and
133 pumps convey the discharge to reservoirs located at ground level. Water is pumped directly from those
134 reservoirs into the network, without further reservoirs located at higher altitude being necessary. The

135 hydraulic head is therefore generated by the pumping stations, the action of which balances the effects
136 of water demand, as suspended reservoir are not present.

137 The pipelines have a total length of 2200 km. The network comprises 31 pumping stations (at the
138 moment 26 are working and connected to the main network, and therefore inserted in the model), and
139 a number of pumps (their number being in the range 3-5) are installed in each of them; each pump
140 works with a discharge in the range 200-400 l/s and with a maximum head of 50 m. On the whole, 96
141 pumps are currently working connected to the main network. Most of the pumps work with a fixed
142 engine speed, but some of them are equipped with Adjustable Speed Drive which enable pumps to
143 work at different velocities. Currently, the network is managed with traditional and empirical
144 techniques.

145 The first step of the study was building the model of the network in order to evaluate its behaviour and
146 verify whether the knowledge about the system is sufficient for its adequate representation, if
147 necessary through the calibration of the model itself. We are perfectly aware that it is simply not
148 possible to represent such a network in all its details and therefore there will be always some model
149 descriptions which can be judged as “rough” or “not adequate”. However, to show the potential of the
150 applied model and to improve the efficiency of the studied system the representation adopted seemed
151 to be appropriate.

152 The available data were the geometry (topology of the network, lengths, diameters, materials, etc.), the
153 mean daily water demand, the pumping schedule for each station, the values of pressure and discharge
154 downstream of each station and in some other junctions quite uniformly distributed within the
155 network. Figure 2 presents a scheme of the whole network.

156 These data were used to build a simplified model of the water supply network using the well-known
157 Epanet software. Simplifications consisted in the insertion in the model only of pipes with a diameter
158 larger than 300 mm. This simplifications was necessary to reduce the time required to build the model
159 and to perform the simulations. However, it was possible to produce a fairly accurate reconstruction of
160 the network, which model is made of 4964 junctions, 96 pumps, and 26 stations, with an overall length
161 of pipes equal to 460 km. As mentioned, five pumping stations have been neglected because not
162 directly connected to the main system or not working.

163 The network was simulated for an average day using the daily average hourly demand curve with a
164 time step of 5 minutes.

165 Figure 3 shows the comparison between simulation results and real data recorded in a pumping station
166 (Baggio, positioned in the SW of the town) and at some points of the network equipped with
167 appropriate instrumentation. As can be seen, the simulated pressures are, in most cases, above the
168 recorded values, and this has been investigated with field tests.

169 **4. Field tests**

170 Some tests were performed at one of the pumping stations in the Milano network, in order to evaluate
171 the performance of the real system and the robustness of the model. The station chosen, named
172 Salemi, is located in the North of the town and it is equipped with four pumps, two with Adjustable
173 Speed Drives and two without such devices. The curves of the pumps were reconstructed in order to
174 be compared with the theoretical curves provided by the system manager and used as inputs to the
175 Epanet model; the result was a good adaptation. Field tests also revealed that fixed speed pumps
176 installed in the Milano water supply network are oversized, as can be seen in figure 4. When the speed
177 of the pump is 100%, the working point is positioned somewhat “to the right” of the curve; the
178 efficiency of the pumps falls dramatically and it is often below 60%. Only when the velocity is
179 reduced the working point moves “to the left” and the efficiency increases. To avoid cavitation, the
180 managers of the network have to partially close the valves positioned downstream the pumps,
181 especially in the morning when the pumps are turned on quite early and their use is slowly requested
182 by the users. These valves induce headlosses which rise power consumption. However, as can be seen
183 in figure 4, the speed reduction which can be obtained with the Adjustable Speed Drives, increases the
184 efficiency of the pumps.

185 The presence of such valves, not included in the model, explains the differences between recorded and
186 simulated data. However, in the paper it has been decided not to insert those valves, as they are not
187 good management of the system; moreover, in the model a fixed efficiency equal to 0.75 has been
188 assigned, because lower values mean that the selected pumps are inappropriate for the system.

189 **5. Results**

190 The results revealed a weak relation between the population size and the number of iterations;
191 generally speaking, the optimum is reached when the individuals are few tens only, while the required
192 number of iterations is even smaller. In this job, the number of individuals has been set equal to 100,
193 which is quite small, if compared with the number of the variables to be set (equal to the number of
194 pumps, and therefore 96) while, applying the rule-of-thumb of a number of individuals equal to five
195 times the number of parameters, convergence should be expected overcoming 500 individuals.
196 However, in Table 1 results are reported for 7:00 a.m. and different numbers of individuals and
197 iterations. As can be seen, the best result is improving when increasing the number of individuals, but
198 that improvement is quite small (1.3% passing from 50 individuals to 100, and 2.8% passing from 100
199 individuals to 500). No improvements can be ascribed to the increasing of the number of iterations;
200 this is applicable to all the other hours: Figure 5 shows the obtained power (W) trend for each
201 configuration obtained through the simulations, carried out for 9:00 p.m. and, as can be seen, the value
202 of W decreases, becoming steady after about 20 thousand runs, but with the minimum reached after
203 only 1500 iterations that, for a population of 100 individuals, means 15 generations.

204 The fast convergence can be ascribed to the fact that the variables to be set (i.e. the velocity of pump
205 functioning) are not continuous and they can assume only fixed (predetermined) values, and therefore
206 the possible solutions fall in a limited field.

207 As mentioned, simulations have been performed considering the efficiency of the pumps is equal to
208 0.75, which is larger than the values computed for the actual pumps, but it has been used in order to
209 have a more consistent comparison between the different scenarios and to carry out results which can
210 be considered more general. For the case of Milano, the consequence is obviously that the expected
211 benefits deriving from the installation of the Adjustable Speed Drivers and their optimization is greater
212 than those computed.

213 Table 2 presents a summary of the results, considering the best configuration (individual) found for
214 each hour of the day. As can be seen, it is possible to save up to 18.9% of energy (on average on one
215 day) with fixed velocity pumps and up to 30.2% of that energy using Adjustable Speed Drivers.

216 In particular, the simulations performed using Adjustable Speed Drivers show that a significant
217 improvement is possible: the minimum saving is 17.1% (at 5:00 a.m.) and the maximum is 38.8% (at
218 10:00 p.m.). Meanwhile, improvements related to the system without these devices have a minimum
219 of 7.6% (at 8:00 a.m.) and a maximum of 29.2% (at 2:00 a.m.).
220 Moreover, to be noted is that in all cases the speed of the pumps never decrease below 70%, because
221 below that value the head of the pump is not sufficient to provide discharge to the network. The
222 required power over the day is summarized in figure 6 for the actual case and for the two
223 optimizations (with and without the Adjustable Speed Drivers).
224 Finally, in order to gain an idea of the possible economic savings, the energy cost is set equal to 0.10
225 €/kWh for the entire day. Therefore, considering the actual total energy in a year equal to 38,200,000
226 kWh, without Adjustable Speed Drivers it is possible to save up to 6,840,000 kWh/year, which means
227 an economic saving of the order of 684,000 € with the use of those devices it is possible to save up to
228 11,627,000 kWh/year, which means an economic saving of 1,162,700 €

229 **6. Conclusions**

230 The paper described the application of Genetic Algorithms to optimize the functioning of the pumping
231 stations of the city of Milano (Italy).
232 To this end, the model of the water supply network was implemented in the well-known Epanet
233 software, and the results of the simulations were checked against real data.
234 Thereafter, thousands of different pumping configurations were tested by means of a simple Genetic
235 Algorithm in order to find the best configuration, i.e. the configuration able to guarantee the same
236 today service and the best energy savings. The configurations were tested with and without the use of
237 Adjustable Speed Drivers. The results showed that a dramatic improvement is possible, both with and
238 without the use of these devices.
239 The results may require further refinement owing to the simplifications adopted for the model, but
240 they are nevertheless encouraging because of the savings that can be achieved in terms of both energy
241 and costs.

242 Field tests showed that the pumps installed in the Milano stations are probably oversized, and
243 therefore the above mentioned values can be increased with a more appropriate choice of the pumps.
244 So far, GA have proved to be an appropriate method for optimization purposes, with many parameters
245 involved and with constraints. As well known, these methods do not allow to find the “best” solution,
246 but a solution which is close to the maximum. Therefore, although good results have been achieved,
247 refinements might be required to further improve the solution. Moreover, methods based on
248 “constructive metaheuristics”, as PSO which has been found promising in Becciu et al. (2015), should
249 be tested to take advantage of their capabilities.
250 Moreover, further research is necessary to combine the energy savings with the different requirements
251 of a complex network like Milano’s, which targets might be contradictory and therefore it may be
252 required to move on the Pareto boundary.

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308

309

Individuals	50	100	500	500
Iterations	100	100	100	500
# Run	Power	Power	Power	Power
1	318404	315949	314083	311146
2	319605	315641	307342	304444
3	317896	315597	307875	313530
4	316469	316574	309452	312418
5	319605	313486	310333	310123
6	319605	312243	310139	311367
7	319605	315640	312134	308330
8	317871	316469	307748	313426
9	315641	311533	309895	304454
10	316080	312473	302695	312189

310

Table 1: Energy required: results of the simulations for 7 a.m. with different GA parameters.

311

312

Hours	0	1	2	3	4	5	6	7	8	9	10	11
% savings fixed speed	17.1	23.3	29.2	28.3	24.2	12.5	11.1	16.4	7.6	9.5	11.5	17.5
% savings adjustable speed	30.0	36.8	36.3	33.0	28.7	17.1	20.2	18.8	26.8	26.1	37.8	31.4
Hours	12	13	14	15	16	17	18	19	20	21	22	23
% savings fixed speed	18.5	21.8	17.1	19.1	19.0	21.2	18.8	22.2	20.4	18.3	21.3	27.4
% savings adjustable speed	32.8	30.1	33.1	34.8	37.4	28.0	24.8	29.2	28.3	34.7	38.8	30.6

313

Table 2: Energy saved, in percentages, with and without the use of inverters for each hour of the average day.

314