# Uncertainty quantification and Global Sensitivity Analysis of subsurface flow parameters to gravimetric variations during pumping tests in unconfined aquifers

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**Key Points:** 

- Effect of hydrogeological parameter uncertainty on drawdown, water content and gravity changes during pumping tests in unconfined aquifers
- The strength of the relative contribution of saturated and unsaturated zone parameters to gravimetric variations markedly varies over time
- Gravimetric information are mostly sensitive to specific yield and aquifer specific storage, especially at early pumping times

#### 1 Abstract

We study the contribution of typically uncertain subsurface flow parameters to gravity 2 3 changes that can be recorded during pumping tests in unconfined aquifers. We do so in the 4 framework of a Global Sensitivity Analysis and quantify the effects of uncertainty of such 5 parameters on the first four statistical moments of the probability distribution of gravimetric 6 variations induced by the operation of the well. System parameters are grouped into two main 7 categories, respectively governing groundwater flow in the unsaturated and saturated portions 8 of the domain. We ground our work on the three-dimensional analytical model proposed by 9 Mishra and Neuman (2011), which fully takes into account the richness of the physical 10 process taking place across the unsaturated and saturated zones and storage effects in a finite 11 radius pumping well. The relative influence of model parameter uncertainties on drawdown, moisture content and gravity changes are quantified through (a) recently developed indices 12 quantifying the relative contribution of each uncertain model parameter to the (ensemble) 13 mean, skewness and kurtosis of the model output, and (b) the Sobol' indices, derived from a 14 15 classical decomposition of variance. Our results document (i) the importance of the effects of the parameters governing the unsaturated flow dynamics on the mean and variance of local 16 17 drawdown and gravity changes; (ii) the marked sensitivity (as expressed in terms of the 18 statistical moments analyzed) of gravity changes to the employed water retention curve model 19 parameter, specific yield and storage, and (iii) the influential role of hydraulic conductivity of 20 the unsaturated and saturated zones to the skewness and kurtosis of gravimetric variation distributions. The observed temporal dynamics of the strength of the relative contribution of 21 22 system parameters to gravimetric variations suggest that gravity data have a clear potential to 23 provide useful information for estimating the key hydraulic parameters of the system.

#### 24 **1. Introduction**

Pumping tests are typically designed and implemented to enhance our ability to 25 characterize aquifer systems. They provide valuable information about hydrodynamic 26 27 parameters (e.g., permeability and/or storage) through the analysis of the system response. The latter is usually considered in terms of drawdown, which represents the variation of 28 hydraulic head at a given point due to pumping. Analytical solutions as well as numerical 29 methods have been proposed by several authors to describe and interpret pumping test 30 31 responses to improve hydrogeological description of a tested system. These include, e.g., the works of Theis (1935), Hantush (1964), Neuman (1972, 1974), Moench (1997a), Raghavan, 32 (2004), Tartakovsky and Neuman (2007), Moench (2008), Mishra and Neuman (2010). In this 33 context, it is recognized that characterizing aquifer parameters by constraints associated with 34 pumping test data is not obvious or trivial. For example, it is known that under some 35 36 conditions, storage and hydraulic conductivity (or transmissivity) can be estimated through 37 pumping responses at short and long times, respectively. Depending on the pumping rate and 38 aquifer hydrogeological setting, the extent of time period within which pumping test data can 39 provide useful information to assess storage can be remarkably variable, thus hampering our ability to optimize the design of a pumping test to fully exploit the information content 40 41 encapsulated in drawdown data.

In this context, estimation of hydrological parameters can benefit from the joint use of hydrological and geophysical information. Geophysical investigations are typically noninvasive and can provide information associated with a large volume of the aquifer system under investigation. Methods which are commonly employed include ground-penetrating radar (Bevan et al., 2003), self-potential responses (Rizzo et al., 2004; Straface et al., 2007), or electrical resistivity imaging (Chang et al., 2017.). Among the sets of geophysical data which can be of interest, gravimetric measurements are increasingly considered to carry

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49 valuable information to effectively complement drawdown data for aquifer characterization. Monitored gravity variations have been shown to embed a remarkable information content 50 and are employed in several applications, including, e.g., geothermal energy (Hunt, 1977; 51 52 Hunt and Bowyer, 2007; Hunt and Graham, 2009; Sofyan et al., 2011; Hinderer et al., 2015) or petroleum engineering (Alnes et al., 2008; Eiken et al., 2008; Young and Lumley, 2015; 53 Kabirzadeh et al., 2017; Katterbauer et al., 2017). Local variations in the acceleration of 54 gravity are due to the Newtonian attraction and to deformations created by loads/stresses. As 55 such, they are linked to a variety of causes, including variations of loading due to 56 displacement of masses of water, as in the cases of, e.g., oceans and atmospheric masses or 57 displacement of fluids in the subsurface. In the context of subsurface hydrology, gravity 58 changes of the order of several µGal have been documented (Damiata and Lee, 2006; Jacob et 59 al., 2008a). These can be detected by modern gravimeters, which can have a resolution of the 60 61 order of the µGal (corresponding to about 5cm of water table variation (Jacob et al., 2008a)). Absolute gravimeters are widely used in hydrology and have the advantage of being (a)62 63 readily transported and (b) non-invasive, so that one can measure variations of gravity at several points in space. 64

The study of Montgomery (1971) is considered as one of the first documented 65 applications of gravimetric data to a hydrological setting, its main target being the estimation 66 of storage of a sandy aquifer in Arizona. Since then, the use of the technique in hydrology 67 applications has gained popularity. Notable examples include the large scale study GRACE 68 (Gravity Recovery and Climate Experiment), where data provide improved understanding of 69 70 water mass variations with a resolution of about 500 km (Tapley et al., 2004; Andersen and Hinderer, 2005; Andersen et al., 2005). Gravity data have also been used for (a) the 71 72 characterization of aquifers located in arid regions (Andersen and Hinderer, 2005; Hinderer et al., 2009; Pfeffer et al., 2011); (b) the study of aquifer recharge, eventually in the context of 73

injection tests (Hunt, 1977; Pool, 2005, 2008; Gehman et al., 2009); (c) the characterization of
karstic aquifers (Jacob et al., 2008b, 2009, 2010; Wilson et al., 2012); and (d) the estimation
of hydrodynamic parameters (Pool and Eychaner, 1995; Naujoks et al., 2010; Christiansen et
al., 2011).

A few recent studies are focused on the analysis of the variation of gravity which 78 could be observed during pumping tests in unconfined aquifers. Damiata and Lee (2006) 79 show that gravimeters have the potential of detecting the effects of variations in hydraulic 80 81 heads caused by a pumping well and rendering estimates of hydrodynamic parameters. Blainey et al. (2007) show that our ability to estimate hydrodynamic parameters of an aquifer 82 is enhanced through a joint use of direct drawdown and gravimetric data. These two 83 preliminary works are limited to fully penetrated wells operating in homogeneous and 84 isotropic aquifers. Herckenrath et al. (2012) extend the results of these studies by considering 85 86 aquifers with anisotropic conductivity where partially penetrating wells are operating. These authors based their analysis on the analytical solution of Moench (1997b), which is employed 87 88 to describe head drawdown. This analytical solution does not explicitly take into account 89 effects due to (a) the presence of an unsaturated region that might overlay the groundwater table prior to pumping, and (b) the system dynamics in the portion of the aquifer which is 90 subject to dewatering during pumping, the rate of drainage from the unsaturated zone being 91 92 modeled as a boundary condition at the water table.

Our work is specifically targeted to the analysis of the gravity changes that can be observed during a pumping test in an unconfined aquifer. Due to the importance of the impact of the unsaturated zone on head drawdowns documented by detailed field experiments (Bevan et al., 2003), numerical studies based on analytical solutions (Mishra and Neuman, 2011) or numerical analyses (Delay et al., 2012), we ground our study on the very recent threedimensional analytical solution proposed by Mishra and Neuman (2011). The latter fully takes 99 into account the effects of the flow dynamics across the unsaturated and saturated zones and 100 the features of the pumping well, which is characterized by a finite radius and storage. Gravity 101 changes induced by the drawdown caused by pumping are quantified through the method 102 proposed by Leirião et al. (2009).

103 Starting from the recognition that model parameters are typically uncertain, the distinctive aim of our study is the assessment of the sensitivity of the hydrodynamic model 104 parameters of the groundwater system to (a) local drawdown, (b) variation of moisture 105 106 content, and ultimately (c) gravity changes induced by pumping. In this context, model parameters can be conceptualized as random variables, and their uncertainty can then 107 108 propagate to target model outputs. As such, the analyses we illustrate contribute to assess the relative importance of uncertain model parameters on statistical moments of the model output 109 of interest. They are also conducive to the assessment of the degree of information content 110 111 embedded in hydrological and gravimetric information of the type we consider.

112 While previous studies have concluded that some of these parameters can be identified 113 using gravimetric variations, no study has considered a complete solution of the flow scenario 114 of the kind we analyze. Blainey et al. (2007) study the contributions of gravity measurements to hydraulic parameter estimation and performed local sensitivity analyses for a given virtual 115 116 setup. Herckenrath et al. (2012) analyze the effect of coupling magnetic resonance sounding 117 and gravity data monitored during a pumping test for the identification of aquifer parameters through inverse modeling. These studies are based on the model developed by Moench 118 Barlow and Moench (1999) and Moench, (1996, 1997). As such, the assessment of 119 120 hydrodynamic parameter identifiability was only limited to saturated hydraulic conductivity and specific yield. 121

122 Our study differs from previous works in terms of (*i*) the richness of the physical 123 processes included in the analytical model employed and (*ii*) the type of sensitivity analysis

we perform. With reference to the latter aspect, we frame our study in the context of a Global 124 Sensitivity Analysis (GSA) approach, recent studies and reviews on this methodology being 125 illustrated by, e.g., Pianosi and Wagener (2015) Razavi and Gupta, (2015) Sarrazin et al. 126 127 (2016). Our GSA is then complemented by the quantification of the way the uncertainty of model parameters propagates to model outputs, i.e., temporal dynamics of local drawdown 128 and moisture content as well as gravity changes. We aim at answering the following research 129 questions: which model parameters are most influential to drawdown, moisture content and 130 131 (local and/or global) gravity changes? At which times? We answer these questions by grounding our GSA on the recent work of Dell'Oca et al. (2017), who propose a set of indices 132 133 that quantify the relative contribution of each uncertain model parameter to the (ensemble) mean, skewness and kurtosis of the model output, and on the Sobol' indices (e.g., Sobol, 134 (1993)), derived from a classical decomposition of variance. 135

The work is organized according to the following structure. Section 2 recalls the main assumption underlying the flow model we rely upon and the link between drawdown and gravity changes in the unsaturated and saturated zone. Section 3 illustrates briefly the GSA we perform and the associated indices. Our results are discussed in Section 4, where we quantify the contribution of the uncertainty associated with each model parameter to the average and variance of drawdown, moisture content and gravity changes during a pumping test.

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#### 143 **2. Theoretical framework**

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#### 2.1 Groundwater table drawdown during a pumping test

We describe drawdown in an unconfined aquifer subject to pumping by way of the recent analytical solution developed by Mishra and Neuman (2011). The latter considers a partially penetrating well and takes into account the presence of an unsaturated zone initially 148 located above the water table as well as the dynamics of flow within the portion of the aquifer149 that is de-saturated during pumping.

A compressible aquifer of infinite lateral extent is considered. The aquifer is assumed 150 to be homogeneous and anisotropic,  $K_r$  and  $K_z$  respectively denoting horizontal and vertical 151 saturated hydraulic conductivities. The water table is initially located at elevation z = b. 152 Pressure head  $\psi$  at the water table corresponds to atmospheric pressure, i.e.,  $\psi = \psi_a$ , and is 153 typically set to 0.0. The initial thickness of the unsaturated zone is denoted as L, ground 154 surface being located at elevation z = b + L. A sketch of the system geometry is depicted in 155 Figure 1. Hydraulic head in the unsaturated zone is initially uniform and equal to  $h_0 = b + \psi_a$ . 156 A pumping well penetrates the aquifer and is screened between elevations *l* and *d* (see Figure 157 1). The pumping rate Q at which the well is operated is uniform in time. The equation 158 describing the water movement in the saturated zone can then be written in cylindrical 159 coordinates as: 160

161 
$$S_{s}\frac{\partial s}{\partial t} = K_{r}\frac{1}{r}\frac{\partial}{\partial r}\left(r\frac{\partial s}{\partial r}\right) + K_{z}\frac{\partial^{2}s}{\partial z^{2}}$$
(1)

#### 162 $S_s$ being specific storage. Drawdown s is given by

163 
$$s(r, z, t) = h(r, z, 0) - h(r, z, t)$$
 (2)

164 h being hydraulic head at elevation z, time t and radial distance r from the well.

165 The initial and boundary conditions associated with (1) are

$$s(\infty, z, t) = 0$$
  

$$\frac{\partial s}{\partial z} = 0 \quad z = 0$$
  

$$\lim_{r \to 0} r \frac{\partial s}{\partial r} = 0 \quad 0 \le z \le b - l \quad b - d \le z \le b$$
  

$$\lim_{r \to 0} r \frac{\partial s}{\partial r} = -\frac{Q}{2\pi K_r (l - d)} \quad b - l \le z \le b - d$$
(3)

166

167 Flow in the unsaturated zone is described by the Richards' equation (Richards, 1931), i.e.,168 following Tartakovsky and Neuman (2007).

169 
$$C_0(z)\frac{\partial\sigma}{\partial t} = K_r k_0(z)\frac{1}{r}\frac{\partial}{\partial r}\left(r\frac{\partial\sigma}{\partial r}\right) + K_z\frac{\partial}{\partial z}\left(k_0(z)\frac{\partial\sigma}{\partial z}\right) \qquad b < z < b + L$$
(4)

170 Here,  $\sigma$  is drawdown in the unsaturated zone, given by

171 
$$\sigma(r, z, t) = h_0 - h(r, z, t) = b + \psi_a - h(r, z, t)$$
(5)

172  $C_0(z)$  is the specific moisture capacity defined as  $C_0(z) = C(\theta_0)$  ( $\theta$  being water content, the 173 subscript 0 indicating the initial conditions), and  $k_0(z)$  the relative hydraulic conductivity. 174 Note that both  $C_0(z)$  and  $k_0(z)$  are not depending on the radial distance from the well.

175 Equation (3) is complemented by the following initial and boundary conditions

176  

$$\begin{aligned}
\sigma(\infty, z, t) &= 0 \\
\frac{\partial \sigma}{\partial z} &= 0 \quad z = b + L \\
\lim_{r \to 0} r \frac{\partial \sigma}{\partial r} &= 0 \quad b \le z \le b + L
\end{aligned}$$
(6)

177 The aquifer water retention curve is represented as (see Mishra and Neuman (2011))

178 
$$S_e = \frac{\theta(\psi) - \theta_r}{S_Y} = e^{a_c(\psi - \psi_a)} \quad a_c \ge 0$$
(7)

179 where  $a_c$  is a model parameter,  $\theta(\psi)$  is water content,  $S_e$  is effective saturation, 180  $S_r = \theta_s - \theta_r$  is specific yield, and  $\theta_s$  and  $\theta_r$  respectively are water content at saturation and 181 residual water content.

182 The Gardner exponential model (Gardner et al., 1958) is used to characterize relative183 hydraulic conductivity, i.e.,

184 
$$k\left(\psi\right) = \begin{cases} e^{a_{k}\left(\psi-\psi_{k}\right)} & \psi \leq \psi_{k} \\ 1 & \psi < \psi_{k} \end{cases} \quad a_{k} \geq 0 \tag{8}$$

185  $a_k \neq a_c$  and  $\psi_k \neq \psi_c$  being model parameters. The parameter  $\psi_k \leq 0$  is usually the air entry 186 pressure head and represents the pressure head above which  $k(\psi)$  is effectively equal to 187 unity.

188 Coupling of the flow across saturated and unsaturated zones is achieved by assuming 189 that pressure is continuous at and flux is normal through the water table. Equations (1) and (4) 190 are thus coupled by way of

191  

$$\frac{\partial s}{\partial z} - \frac{\partial \sigma}{\partial z} = 0 \qquad z = b$$
(9)

192 Mishra and Neuman (2010) write the drawdown in the saturated zone as

$$s = s_H + s_U \tag{10}$$

Here,  $s_U$  is the component of the drawdown accounting for the contribution of the unsaturated zone on the water table fluctuation; and  $s_H$  is a modified Hantush solution (Hantush, 1964). Whereas the Hantush solution describes flow towards a partially penetrating well of zero radius in a confined aquifer, the modified solution introduced by Mishra and Neuman (2011) accounts for storage effects in a partially penetrating pumping well with finite radius  $r_w$  and storage coefficient  $C_w$ .

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#### 2.2 Gravity variations due to groundwater table drawdown

Gravimetric variations within a time interval  $\delta t$  are due to change in the water content, expressed in terms of mass, in the domain. Considering a cylindrical coordinate system, the following formulation can be employed to quantify such variations, as detected by a gravimeter located at ( $r_m$ ,  $z_m$ ) within a domain of infinite extent (Telford et al., 1990)

206 
$$\Delta g = \int_{-\infty}^{+\infty} \int_{-\infty}^{+\infty} \gamma \Delta \rho(r, z) \frac{-(z - z_m)}{\left(\left(r - r_m\right)^2 + (z - z_m)^2\right)^{3/2}} dz dr$$
(11)

Here,  $\Delta g$  (L T<sup>-2</sup>) is the variation of gravity (or gravity change) between time *t* from the beginning of pumping and the initial (undisturbed) conditions and caused by a change of mass at locations associated with radial coordinate *r* and vertical coordinate *z* where a density change  $\Delta \rho$  (M L<sup>-3</sup>) takes place, and  $\gamma = 6.67 \times 10^{-11}$  (N m<sup>2</sup> kg<sup>-2</sup>) is the universal gravitational constant.

212 Density changes  $\Delta \rho$  within a volume  $\Delta \Omega = (\pi (r+dr)^2 - \pi r^2) dz$  depend on the 213 change of (a) water head,  $\Delta h$ , in the saturated zone and (b) water content,  $\Delta \theta$ , in the 214 unsaturated region through

$$\Delta \rho = \rho_w S_s \Delta h \tag{12}$$

216 
$$\Delta \rho = \rho_w \Delta \theta \tag{13}$$

where  $\Delta\theta$  can be evaluated via (7) and  $\rho_w$  is water density, (12) and (13) respectively referring to the saturated and unsaturated regions. The global change in gravity at the scale of the pumping test is then obtained by the numerical integration of (11).

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#### 221 **3. Global Sensitivity Analysis**

As highlighted by (11) - (13), gravity changes depend on a set of hydrogeological 222 parameters. The uncertainty associated with these parameters is typically due to lack of 223 information and is then propagated to state variables of interest, notably to  $\Delta g$ ,  $\Delta h$ , and local 224 moisture content or effective saturation. Global Sensitivity Analysis (GSA) provides a 225 226 theoretical framework within which one can then quantify the influence of these uncertain quantities on key (statistical) moments of target model output quantities. In this context, we 227 228 focus on four sets of indices: (i) the indices introduced by Dell'Oca et al. (2017), and (ii) the 229 Sobol' indices (Sobol, 1993). These indices respectively enable us to quantify the relative 230 contribution of each uncertain model parameter to the mean (expected value), variance, skewness, and kurtosis of the state variable of interest. Having at our disposal this information
enables us to rank model parameters in order of importance with respect to a given statistical
moment of the model output.

234 Performing a GSA requires spanning the entire parameter space and performing multiple runs of the process model of choice in a Monte Carlo framework. In some cases, this 235 might lead to high computational costs, which can hamper the practical feasibility of the 236 237 analysis. It has then become common procedure to approximate the complete system model through a surrogate model. The latter can be considered as a reduced complexity 238 approximation of the original model and can be employed to perform multiple Monte Carlo 239 240 runs with a sufficient accuracy and at an affordable computational time. As noted by Mishra and Neuman (2010, their Appendix C and D), the analytical solution we employ can be 241 computationally demanding. For example, we verified that calculation of the solution at one 242 243 point for the full simulation time can take up to 1 hour to 20 hours on a computer Intel Core i7 3.20GHz, depending on the parameter set values, due to the need for evaluating numerous 244 245 integrals. As a consequence, we resort to a strategy based on the construction of a surrogate 246 model to perform GSA in our study. Amongst available alternatives, we base our GSA on the formulation of a surrogate model based on the Polynomial Chaos Expansion (PCE) 247 framework. The latter has been broadly used to perform GSA in various fields of applications 248 (Sudret, 2008; Crestaux et al., 2009; Fajraoui et al., 2011; Formaggia et al., 2012; Ciriello et 249 al., 2013a; Fajraoui, 2014; Garcia-Cabrejo and Valocchi, 2014; Sudret and Mai, 2015) and 250 251 yields the target global sensitivity indices in a straightforward manner.

We briefly summarize in the following the theoretical elements characterizing the GSA indices we employ and the PCE technique. We refer to appropriate literature for additional details.

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#### 256

#### **3.1.** The AMA indices (Dell'Oca et al., 2017)

As observed by Dell'Oca et al. (2017), a limitation of grounding a GSA solely on the 257 Sobol' indices (see also Section 3.2 for a synthetic illustration of these indices) is that the 258 259 uncertainty of a target model output, y, is considered to be fully characterized by its variance. As such, ranking the relative importance of model parameters upon relying solely on the 260 analysis of Sobol' indices might provide an incomplete picture of a system response to model 261 262 parameters. Here, we also quantify the effects that uncertain model parameters can have on the mean (expected value) of y, to broaden the scope of the GSA we perform. We do so by 263 relying on the metrics introduced by Dell'Oca et al. (2017), i.e., 264

265 
$$AMAE_{x_i} = \frac{1}{|y_0|} \int_{\Gamma_{x_i}} |y_0 - E[y | x_i]| \rho_{\Gamma x_i} dx_i = \frac{1}{|y_0|} E[|y_0 - E[y | x_i]|]$$
(14a)

266 
$$AMA\gamma_{x_i} = \frac{1}{\left|\gamma[y]\right|} \int_{\Gamma_{x_i}} \left|\gamma[y] - \gamma[y \mid x_i]\right| \rho_{\Gamma x_i} dx_i = \frac{1}{\left|\gamma[y]\right|} E\left[\left|\gamma_y - \gamma[y \mid x_i]\right|\right]$$
(14b)

267 
$$AMAk_{x_i} = \frac{1}{k[y]} \int_{\Gamma_{x_i}} |k[y] - k[y|x_i]| \rho_{\Gamma x_i} dx_i = \frac{1}{k[y]} E[|k[y] - k[y|x_i]|]$$
(14c)

Here,  $y_0$ ,  $\gamma[y]$ , and k[y] respectively are the mean, skewness and kurtosis of y, 268  $\Gamma_{x_i} = [x_{i,\min}, x_{i,\max}]$  is the support of the *i*-th random variable  $x_i$  (ranging between  $x_{i,\min}$ , and 269  $x_{i,\max}$ ;  $E[y|x_i]$ ,  $\gamma[y|x_i]$ , and  $k[y|x_i]$  respectively are the mean, skewness, and kurtosis of 270 y conditional on  $x_i$ ; and  $\rho_{\Gamma_{x_i}}$  is the marginal probability density function (*pdf*) of  $x_i$ . Similar 271 272 to the Sobol' indices, we can also evaluate the joint effect of parameters on the mean and therefore the total index associated with a given parameter. Evaluation of the indices (14a)-273 274 (14c) enables us to quantify the expected variation of the corresponding statistical moments of a target quantity due to conditioning on a given system parameter. Relying on these indices 275 provides information on the way features of the probability distribution of y (i.e., mean, 276

symmetry, and tailedness) can be influenced by uncertain model parameters. The reader is
referred to Dell'Oca et al. (2017) for additional details.

279

#### 280 3.2. The Sobol' indices

281 Let us consider the output y of a mathematical model f having n input parameters 282  $(x_1, x_2, \dots, x_n)$ , i.e.,

$$y = f(x_1, x_2, \dots, x_n)$$
 (14)

We assume *f* to belong to the space of square integrable functions and the *n* uncertain input parameters to be defined in  $[0,1]^n$ . The function *f* can be decomposed into sums of polynomials of increasing power, i.e.,

287 
$$f(x_1, x_2, \dots, x_n) = f_0 + \sum_{i=1}^n f_i(x_i) + \sum_{j>1}^n f_{ij}(x_i, x_j) + \dots + f_{1,2,\dots,n}(x_1, x_2, \dots, x_n)$$
(15)

where  $f_0$  is the expected value of f, and  $f_{1,2,\dots,n}(x_1, x_2, \dots, x_n)$  are orthogonal functions.

Decomposition (16) is based on the analysis of variance (ANOVA, Archer et al., 1997) and is unique. By squaring (16) and integrating over  $[0,1]^n$ , we obtain

291 
$$V = \sum_{i=1}^{n} V_i + \sum_{j>1}^{n} V_{ij} + \dots + V_{1,\dots,n}$$
(16)

Here, *V* is the total variance of *y*,  $V_i$  and  $V_{ij}$  respectively being the contribution to *V* due to input  $x_i$  alone and due to the interactions of parameters  $x_i$  and  $x_j$ .

294 The principal Sobol' sensitivity indices (Sobol, 1993) are given by

$$S_i = \frac{V_i}{V} \tag{17}$$

and describe the relative contribution to V due to variability of only  $x_i$ . Note that the principal Sobol' index embeds the relative expected reduction of the variance of y due to knowledge of (or conditioning on) parameter  $x_i$ .

299 Otherwise, the total Sobol' indices

300 
$$S_{i}^{tot} = \frac{\sum_{i \in \{i_{1}, \dots, i_{s}\}} V_{i_{1}, \dots, i_{s}}}{V}$$
(18)

quantify the total contribution of  $x_i$  to V, including all terms where  $x_i$  appears, i.e.,  $S_i^{tot}$  also includes interactions between  $x_i$  and the remaining uncertain parameters.

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#### 304 **3.3. Construction of the surrogate model using polynomial chaos expansion**

Relying jointly on the AMAE (14a), AMA $\gamma$  (14b), AMAk (14c), and Sobol' indices (introduced in Sections 3.2.1 and 3.2.2) enables one to perform a GSA of process *y* quantifying the impact of each of the uncertain model parameters on the first four (statistical) moments of the *pdf* of *y*. This strategy yields information about the way these important elements of the distribution of *y* are impacted by model uncertain parameters. Calculation of these indices entails evaluation of conditional moments of *y* that are here computed using the PCE - based approximation of the full system model.

Following Wiener (1938) and Xiu and Karniadakis (2002), we represent  $f(\mathbf{x})$  ( $\mathbf{x}$  being the vector collecting random system parameters  $x_i$ , i = 1, 2, ..., n) as

314 
$$f(x) = \sum_{j=0}^{+\infty} a_j \zeta_j(x_1, \dots, x_n)$$
(19)

where  $a_j$  are polynomial coefficients and  $\zeta_j(x_1,...,x_n)$  are multivariate orthogonal polynomials which depend on the joint probability function of the random model parameter. For computational purposes, decomposition (19) is truncated to a finite order *M* as

318 
$$y = a_0 \xi_0 + \sum_{j=1}^{M-1} a_j \xi_1(x_j) + \sum_{j \ge i}^{M-1} a_{ij} \xi_2(x_j, x_i) + \sum_{k \ge j}^{M-1} a_{ijk} \xi_3(x_j, x_i, x_k) + \dots \dots$$
(20)

319 where  $M = \frac{(n+p)!}{n!p!}$ , p being the polynomial degree retained for each function  $\xi_i$ .

Coefficients  $a_j$  are calculated through an approach that requires evaluating the full 320 system model at a number of points in the parameter space and then performing least square 321 regressions (Sudret, 2008). We note that the number of coefficients may be prohibitively large 322 when the number of random model parameters increases. Thus, several approaches have been 323 developed to minimize computational cost by appropriate selection of model evaluation points 324 325 in the parameter space (e.g., Blatman and Sudret, 2010b, 2010a, 2011; Fajraoui et al., 2012) and reference therein. Here, we apply the sparse grid sampling technique suggested by 326 Fajraoui et al. (2012). Following this approach, only coefficients whose contribution to the 327 328 output is higher than a user defined threshold are retained, thus reducing the number of full 329 model simulations required to estimate the polynomial coefficients. Sobol' indices are evaluated as the coefficients of the PCE, the AMAE, AMAy, and AMAk indices being 330 computed through Monte Carlo runs of the PCE. 331

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4. Sensitivity of drawdowns, effective saturation and gravity changes to hydrogeological
 parameters during a pumping test

4.1 Problem set-up

We consider an unconfined homogeneous aquifer whose water table is located 10 m below the ground surface and the initial hydraulic head is equal to 50 m. A partially penetrating pumping well is operating in the system. In our example, the well is screened from 39 m to 40 m below the ground surface and is operated at a uniform pumping rate Q = $6.30 \times 10^{-2}$  m<sup>3</sup>/s. The well is characterized by a dimensionless radius  $r_{wD} = r_w/b = 0.02$  and storage  $C_{wD} = C_w/b = 0.10$ . A gravimeter is installed on the surface and at the same position as the pumping well (Figure 1).

343 Drawdowns are computed at a set of radial distances, defined according to a 344 logarithmic spacing, i.e.,

$$r_{1} = \Delta r$$

$$r_{i} = 10^{\log(r_{i-1}) + \Delta r}$$
(21)

where  $\Delta r = 10$  m. At each radial distance, drawdown is also computed along the vertical at a set of elevations arranged according the same logarithmic spacing design as in (21).

348 We simulate the test across 7 days of operation. This duration is consistent with duration of pumping tests in unconfined systems (see, e.g., Bevan et al. (2003) and references 349 350 therein) and allowed to reach pseudo-steady state for the mean drawdown in our study. We 351 also note that the scenario analyzed corresponds to the one presented by Darmiata and Lee (2006) and Leiriao et al. (2009) and can then be considered as a proxy for a field scale test, in 352 353 terms of positioning and flow rate of the well, duration of the pumping operation, and range 354 of variability of the system parameters. We perform a GSA of the drawdown, soil moisture and gravimetric variations to the following dimensionless parameters: (a)  $L_D = L/b$ , which is 355 a characteristic (dimensionless) system length scale; (b) the anisotropy factor  $K_D = K_z / K_r$ ; 356 (c) the specific storage of the saturated zone  $S_s$ ; (d) the specific yield  $S_y$ , (e)  $a_{cD} = a_c b$  and 357  $a_{kD} = a_k b$ , which are respectively associated with the parameters used in the water retention 358 and relative hydraulic conductivity functions. 359

Model uncertain parameters are considered as independent and identically distributed (*i.i.d.*) random variables, each characterized by a uniform distribution within the intervals listed in Table 1. These intervals are normalized between (0, 1) for the construction of the PCE. We perform 500 full model simulations within a Quasi Monte Carlo sampling approach, a sampling technique that has desirable convergence properties and is space filling (Feil, 365 2009). PCE models of increasing order were built by considering 400 simulations, randomly 366 selected amongst the total number of simulations performed. The accuracy of the ensuing 367 PCE for drawdowns, soil moisture and gravity changes were evaluated by cross-validations 368 against the remaining 100 simulations. The procedure was repeated by considering various 369 sets of randomly selected simulations for the construction and validation of the PCE. A PCE 370 of order 4 was considered as appropriate in terms of accuracy (details not shown).

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#### 4.2. Results and discussion

We present our results at two scales, i.e., a small scale, representing a volume of the aquifer that can be considered as the measurement scale of heads and moisture content and the global scale of the pumping test, which represents the scale at which pointwise gravity changes are integrated by the gravimeter.

376

# 4.2.1 Temporal variations of drawdown, effective saturation and gravity changes at a local scale

379 We illustrate here the analyses of the sensitivity of our target variables to the selected uncertain model parameters at a local scale. We define the latter as a volume of size 380  $V = \left[ \pi \left( r + \Delta r / 2 \right)^2 - \pi \left( r - \Delta r / 2 \right)^2 \right] \Delta z \text{ with } \Delta r = \Delta z = 10 \text{ m, centered at a given point } A \text{ in}$ 381 the aquifer. For purpose of illustration, we position A at the initial position of the interface 382 between the saturated and the unsaturated zones (i.e., r = z = 10 m). This location has been 383 384 chosen since it is close to the well and enables us to clearly highlight the diverse contributions of parameter uncertainty to the variables of interest, i.e., drawdown, effective saturation and 385 gravity changes. 386

Figure 2a depicts the temporal evolution of the mean (continuous curve) drawdown and its related uncertainty at this location based on 500 runs of the analytical solution. The level of uncertainty is illustrated by the shaded area whose limits correspond to one standard deviation. A corresponding depiction of the temporal dynamics of effective saturation isshown in Figure 2b.

The observed evolution of the mean drawdown imbues the effects of an artesian storage during early times (until about 3000 s from the beginning of pumping) and drainage from the unsaturated zone during late times.

The effective saturation,  $S_e$ , can also be directly measured in the field and represents the variations of the water content in the unsaturated zone. As expected, the mean effective saturation of the considered volume decreases with time. It is noted that there is a very significant impact of the parameter uncertainty, as quantified by the variance of  $S_e$ .

The corresponding temporal dynamics of gravity changes detected between the initial 399 (undisturbed) condition and time t are due to the temporal variation of mass of water in the 400 401 volume considered and are depicted in Figure 3. Note that here and in the following we denote gravity change calculated at time t as the difference between gravity at t and at the 402 403 initial system state. These changes range on average between 0.0 and 0.5  $\mu$ Gal, and can attain values as large as 2 µGal at late times. We note that, as stated above, these results are 404 associated with a local scale volume that is in the vicinity of the well and of the ground 405 surface, where the gravimeter is positioned, so that the drawdown taking place within it 406 markedly contributes to the gravity change detected by the gravimeter. Comparison of Figures 407 2 and 3 suggests that the variance of gravity changes,  $\Delta g$ , is larger and increases at a higher 408 409 temporal rate than that of drawdown,  $\Delta h$ . This is related to the structure of (11)-(13), from 410 which it can be seen that a random gravity change is proportional to the product of two 411 (correlated) random quantities, i.e.,  $S_s$  and  $\Delta h$  in (12) or  $\Delta \theta$  in (13) the latter, in turn, depending on  $S_{\gamma}$ ,  $\Delta h$  and  $a_{cD}$ . 412

Figure 4 depicts the contribution of the uncertainty of each model parameter to the mean (i.e., in terms of AMAE indices (14a) in Figure 4a) and to the variance (i.e., in terms of

Sobol' indices in Figure 4b) of drawdown. These results show that the specific storage  $S_s$  is 415 the main parameter governing the mean and variance of drawdown during the first hours of 416 pumping (up to approximately 3,000 s). The uncertainty related to the anisotropic factor  $K_D$ 417 has an essentially uniform contribution to the average drawdown (Figure 4a) after 30,000 s; it 418 contributes significantly to drawdown variance (Figure 4b) between time t = 3,000 s and 419 100,000 s, as compared to the parameters related to the unsaturated zone (i.e.,  $S_Y$ ,  $a_{kD}$ , and 420  $a_{\rm cD}$ ). Contributions of the parameters characterizing the unsaturated zone appear to be non-421 422 negligible only at late times, when the contribution of the parameters related to the saturated zone becomes of secondary importance. 423

424 The sensitivity of the drawdown to the unsaturated zone parameters tend to increase with time, while the contribution of the specific storage is observed to acquire lesser 425 importance. This is due to the effects of artesian storage taking place during early pumping 426 427 times. It can be observed that the sensitivity of the specific storage to the mean drawdown starts decreasing as soon as pumping starts (Figure 4a), its sensitivity to drawdown variance 428 remaining constant during the first minutes of pumping (Figure 4b). The mean and variance of 429 the drawdown are insensitive to the initial thickness of the unsaturated zone,  $L_D$ . This is 430 consistent with the conclusions of Mishra and Neuman (2011), who pointed out that the initial 431 unsaturated zone thickness (when greater than one quarter of the saturated thickness) has no 432 433 significant effect on the drawdown. The drawdown in the saturated zone depends solely on 434 the unsaturated flow dynamics taking place close to the water table.

The parameters used to model flow in the unsaturated zone,  $a_{cD}$  and  $a_{kD}$ , attain the highest importance for the longest observation times, corresponding to the drainage of the unsaturated zone. At late pumping times, the most significant contributions to the mean and variance of drawdown are due to the uncertainty related to  $a_{cD}$  and  $a_{kD}$ . Hydraulic

conductivity of the unsaturated zone decreases rapidly with pressure for high values of  $a_{\rm kD}$ 439 (see (8)), thus causing an increase of the drawdown in the saturated zone, because the 440 unsaturated zone provides less water. Very large values of  $a_{kD}$  lead to a virtually 441 impermeable unsaturated zone. The unsaturated zone loses its ability to store water above the 442 water table also for large values of  $a_{cD}$ , causing an increase of the contribution of the 443 444 unsaturated zone to the drawdown (drainage) and therefore, the drawdown decreases at the beginning of the pumping test. The capacity of the unsaturated zone to store water increases 445 446 when  $a_{cD}$  is small, this scenario causing delayed water table response and drawdown at the beginning of the pumping test. 447

Figure 5 depicts the temporal evolution of both sets of GSA indices evaluated for 448 effective saturation  $S_e$  within the same sample volume corresponding to Figure 4. The water 449 retention parameter  $a_{cD}$  contributes in very distinct ways to the mean (Figure 5a) or to the 450 variance (Figure 5b) of the effective saturation, i.e., its contribution increasing or being 451 approximately uniform in time for the mean and for the variance. The high sensitivity of  $a_{\rm cD}$ 452 is consistent with the observation that it quantifies the amount of water released for a given 453 pressure drop (see (7)). The opposite behavior is documented for the specific storage  $S_s$ , 454 whose contribution remains constant for the mean and decreases with time for the variance. 455 Similar to the drawdown, the effective saturation is sensitive to  $S_s$  solely during the early 456 time of pumping. 457

Variability in gravity changes are mainly controlled by the specific yield  $S_{Y}$ , the specific storage  $S_{S}$ , and the water retention curve parameter  $a_{cD}$  (Figure 6). The relative contribution of conductivity anisotropy and unsaturated zone parameters ( $L_{D}$  and  $a_{kD}$ ) to the mean gravity changes is significant. This is clearly seen in Figure 6a, where these parameters

are seen to be associated with sensitivity indices which are almost constant with time and 462 463 greater than 0.25. The influences on the variance of the gravity changes (Figure 6b) of the parameters are negligible (with total Sobol' indices less than 0.05) except for the parameters 464 related to water storage (i.e., specific yield and specific storage) and  $a_{cD}$ . Unlike the 465 drawdown, we found that the mean gravity changes are slightly sensitive to the initial 466 thickness of the unsaturated zone. Gravity changes depend on drawdown, distance from the 467 gravimeter, the specific yield and the parameter  $a_{cD}$  associated with the dynamics of the 468 unsaturated zone, as well as on the specific storage of the saturated zone. Therefore, gravity 469 470 changes due to pressure head variations in the saturated zone are significantly smaller than 471 those due to pressure head variations in unsaturated zone.

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#### 473 **4.2.2 Total gravity changes at the pumping test scale**

The gravimeter yields a measure of the gravity changes occurring throughout the 474 475 whole region affected by pumping. Note that, according to (11), the contribution of a given point in the aquifer (that can be considered as the centroid of a given measurement volume of 476 the kind explored, e.g., in Section 4.2.1) is weighted by the square of its inverse distance from 477 478 the gravimeter. Figure 7 depicts the evolution with time of the mean gravity change detected 479 over the whole domain (Figure 7a) and of the sample probability density functions of gravity 480 changes (Figure 7b) associated with three selected observation times (i.e., 100 s, 4 h, and 7 481 days). These results show that the mean and variance of the global variations of gravity at the 482 scale of the pumping test display a trend which is similar to that observed at the local scale (compare Figures 7 and 3). The largest mean value is approximately equal to  $1.14 \mu$ Gal and is 483 484 obviously attained at the end of the pumping period, where a quite large variance is also observed (the variance is equal to  $1.3 \,\mu \text{Gal}^2$ , the associated coefficient of variation being 1). 485 The resulting sample probability density function at a given observation time can be 486

interpreted through an Exponential distribution, as shown in Figure 7b, the corresponding 487 scale parameter coinciding with the mean value depicted in Figure 7a. Close inspection of the 488 sample probability densities depicted in Figure 7 reveals that in some regions of the parameter 489 490 space gravity changes at late time (i.e., 7 days) can be significant. For example, they can 491 attain values as large as 5 or 6 µGal with non-negligible probability. Otherwise, probability that total gravity changes be larger than, e.g., 5 µGal is virtually negligible for all practical 492 purposes at early time. These results suggest that, depending on the characteristic system 493 494 parameters, there is a clear potential to discriminate total gravity changes due to the effect of pumping at late time with typical instrumentations. The latter can be associated with 495 sensitivities and accuracy which are compatible with the gravity change values we find, 496 497 depending on conditions (e.g, Merlet et al., 2008; Jacob et al., 2009; Gehman et al., 2009; Christiansen et al., 2011a, b; González-Quirós and Fernández-Álvarez, 2017). As an 498 499 additional comment, we note that in this study we assess total gravity changes measured 500 across the pumping test through a single gravimeter located at the well position. A possible 501 extension of the analysis would entail the use of a network of gravimeters, arranged according 502 to a given pattern. This would be associated with the added value of enhancing the 503 detectability of total gravity changes by taking into account effects of correlations amongst the diverse measurement points (e.g., Gehman et al., 2009; Jacob et al., 2009, 2010; 504 505 Christiansen et al., 2011b; Herckenrath et al., 2012).

Figures 8 depict the temporal evolution of the AMAE (14a), Sobol', AMAγ (14b), and AMAk (14c) indices related to the total change in gravimetry. The general temporal dynamics of the AMAE (Figure 8a) and Sobol' (Figure 8b) indices are essentially similar to those displayed by gravimetric variations at the local scale. Note that the total gravimetric change represents the integral of the local scale changes, thus explaining the observed similarity. Skewness and kurtosis of the detected total gravity changes are essentially influenced by all 512 system parameters throughout the temporal window examined. This suggest that there is a 513 clear potential that global gravity changes data can contribute to the identification of the main 514 system parameters.

515 Figure 9 depicts the spatial distribution of the mean and variance of drawdowns calculated throughout a vertical cross-section (each point being identified by coordinates (r, r)516 z)) at three selected representative times, i.e., t = 100 s (early time behavior), 4 hours 517 518 (intermediate time, where the effects of specific storage decrease), and 7 days (pseudo-steady 519 state). Since we verified that the spatial distributions of the AMAE and Sobol' indices provide very similar information (not shown), our illustrations focus solely on the Sobol' indices 520 (Figure 9). We also observed that the behavior of parameter  $a_{cD}$  (which is associated with the 521 water retention curve) is very similar to the behavior of parameter  $a_{kD}$  (which is involved in 522 the relative conductivity model). Therefore, we do not represent the behavior of  $a_{cD}$  in the 523 following plots. Note that the quality of the graphical depictions depends on the spacing of the 524 points at which the analytical solution has been determined. A finer grid will provide 525 smoother maps, requiring an increased computer time (see Section 3). 526

Figure 9 suggests that the mean drawdown is less than 1 m even close to the well after 100 s of pumping, its associated variance being mainly due to the uncertainty of the specific storage  $S_s$ . The contribution of  $S_s$  to the variance tends to increase at locations close to the well.

Results after 4 hours show that the drawdown is equal to 4 m on average around the pumping well. The sensitivity of  $S_s$  is significantly decreased at this time, as compared to early withdrawal times. Otherwise, we can see that the value of the total Sobol' indices of  $S_Y$ ,  $K_D$ , and  $a_{kD}$  are enhanced with respect to the corresponding early time results. The spatial distribution of the Sobol' indices related to the parameters linked to hydraulic conductivity ( 536  $K_D$  and  $a_{kD}$ ) is very different than that associated with the remaining parameters. The indices 537 are higher close to the well for  $K_D$  and higher far from the well for  $a_{kD}$ .

On day 7 from the beginning of pumping, the mean of the drawdown varies between 4 and 6 m, the highest drawdown being more than 6 m near the well. At this time, the variance of the drawdowns is controlled mainly by the contributions of the parameters related to unsaturated flow (i.e.,  $S_Y$ ,  $a_{kD}$ , and  $a_{cD}$ ) and by the factor of anisotropy ( $K_D$ ).

The contribution of the parameters involved in the unsaturated flow (water retention and relative conductivity) to the drawdown variance increases with time. This implies that the uncertainty of the drawdowns for long times depends on the hydrodynamic behavior of the unsaturated zone. These parameters do not affect drawdowns uncertainty for short times, when the amount of pumped water is mainly linked to the specific storage (see sensitivity of  $S_s$  at time equal to 100 s) and to hydraulic parameters of the saturated zone at the intermediate times (see sensitivity of  $K_D$  at time 4 hours).

The distribution of the mean and variance of the global gravity changes and the related 549 Sobol' indices are depicted in Figure 10. The hydrogeological system parameters that do not 550 551 contribute to the variance significantly and are not included in the figure. Volumetric parameters (i.e., specific storage and specific yield) and the parameter  $a_{cD}$  appearing in (7) 552 are the only contributors to the gravimetric changes variance. Gravity changes at t = 100 s are 553 very small. At 4 hours and 7 days after the beginning of the pumping, the spatial distributions 554 555 of the gravity changes indicate that only the changes of the mass of water within a radius of about 15 m and over a depth less than 15 m contribute to the gravimetric variations (Figure 556 10). 557

558 Close to the surface, gravimetric variations are essentially controlled by the specific 559 yield and  $a_{cD}$ . The sensitivity of the specific storage and specific yield respectively decreases and increases with depth (Figure 10). At some depths (such as, e.g., at point A, as illustrated in Section 4.2.1), these variations are controlled by the effects of both specific storage and specific yield. The sensitivity of the specific storage decreases with time, similar to its impact on the drawdown. Otherwise, sensitivity of the specific yield slightly increases with time.

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#### 565 **5. Conclusions**

566 Our work is focused on the assessment of the strength of the relative contribution of typically uncertain parameters governing flow in variably saturated porous media to gravity 567 changes that can be recorded during pumping tests in unconfined aquifers. We model 568 569 drawdown by way of the fully three-dimensional analytical solution of Mishra and Neuman 570 (2011), which explicitly takes into account flow processes across the unsaturated and saturated zones and storage effects in a finite radius pumping well. Gravimetric variations 571 572 induced by the change of hydraulic head due to pumping and detected by a gravimeter installed at the pumping well location are quantified via the formulations of Telford et al. 573 574 (1990) and Leirião et al. (2009). We base our study on a Global Sensitivity Analysis approach 575 and quantify the effects of the uncertain model parameters on four statistical moments of gravimetric variations associated with pumping. Our work leads to the following major 576 conclusions. 577

578 1. The strength of the relative contribution of saturated and unsaturated zone parameters 579 to the mean and variance of local drawdown, effective saturation, as well as local and 580 global gravimetric variations markedly varies over time. This behavior is quantified 581 through (*a*) recently developed indices (Dell'Oca et al., 2017) quantifying the relative 582 contribution of each uncertain model parameter to the (ensemble) mean, skewness and 583 kurtosis of the model output, and (*b*) the classical Sobol' indices, derived from a 584 decomposition of variance. Our result document that the uncertainty associated with a 585 given model parameter can impact the first four (statistical) moments of the variables 586 analyzed in a different way, as expressed through the set of sensitivity indices we 587 consider.

- 588 2. The mean and the variance of the changes in gravity are mainly controlled by the 589 uncertainty associated with specific yield, the parameter of the water retention curve 590  $a_{cD}$  (7), and aquifer specific storage. All uncertain system parameters considered in 591 the analysis are influential to the skewness and kurtosis, respectively expressing the 592 degree of asymmetry and tailedness of the probability density function of gravity 593 changes.
- 594 3. The mean and the variance of drawdown are sensitive to specific storage solely at the 595 beginning of the pumping test. The most significant contributions to the mean and 596 variance of drawdown at late pumping times are due to the uncertainty related to the 597 parameters driving flow in the unsaturated zone.
- 598 4. Sample probability density functions of total gravity changes can be interpreted 599 through Exponential distributions (see Figure 7). Our results suggest that in some 600 regions of the parameter space gravity changes at late time (i.e., 7 days) can be 601 significant and larger than about 3  $\mu$ Gal, a value corresponding approximately to 602 reported modern gravimeter accuracy.
- 5. The results of our Global Sensitivity Analysis suggest that, under the assumptions associated with the analytical model considered, gravimetric data tend to provide limited contribution for the estimation of hydraulic conductivity in the saturated or unsaturated regions, the variance and the mean of drawdowns being more sensitive to these model parameters. Otherwise, gravity data might contribute to infer estimates of aquifer storage terms and water retention curve parameters. From a practical point of view, coupling gravimetric and drawdown measurements during a pumping test have a

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high potential to yield improved estimates of saturated and unsaturated regions flow
parameters. A natural extension of the study is also related to the assessment of the
way the use of the comprehensive set of sensitivity metrics can complement methods
based solely on the Sobol' indices (e.g., Ciriello et al., 2013b, 2015) for a design of
experiments targeted to prioritize data acquisition for the characterization of specific
features of the probability distribution of a desired variable.

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## Table caption

Table1. Ranges of variability of model parameters.

Parameters	Variability range
$S_{s} (m^{-1})$	$(10^{-5} - 10^{-3})$
S <sub>Y</sub> (-)	(10 <sup>-2</sup> - 0.50)
<i>a</i> <sub>kD</sub> (-)	(2 - 1000)
<i>a<sub>cD</sub></i> (-)	(0.1 - 100)
<i>K</i> <sub>D</sub> (-)	(0.05 – 1.0)
$L_D$ (-)	(0.01 - 0.70)

### **Figure caption**

Figure 1. Schematic representation of system geometry.

Figure 2. Temporal evolution of the mean (continuous curve) (*a*) drawdown and (*b*) effective saturation calculated within a volume centered at the initial position of the interface between the saturated and the unsaturated zones. The width of the shaded area corresponds to one standard deviation.

Figure 3. Temporal evolution of the mean (continuous curve) gravity changes computed between the initial (undisturbed) condition and time *t* within the same volume considered in Figure 2. The width of the shaded area corresponds to two standard deviations.

Figure 4. Contribution of the uncertainty of each model parameter to (*a*) the mean (AMAE Indices) and to (*b*) the variance (Sobol' indices) of drawdown.

Figure 5. Contribution of the uncertainty of each model parameter to (a) the mean (AMAE Indices) and to (b) the variance (Sobol' indices) of effective saturation.

Figure 6. Contribution of the uncertainty of each model parameter to (*a*) the mean (AMAE Indices) and to (*b*) the variance (Sobol' indices) of gravity changes.

Figure 7. (a) Temporal evolution of the mean gravity change (continuous curve) over the whole domain (the width of the shaded area corresponds to two standard deviations) and (b) probability density functions for three selected times

Figure 8. Contribution of the uncertainty of each model parameter to (*a*) the mean (AMAE Index), to (*b*) the variance (Sobol' indices), to (*c*) the skewness (AMA $\gamma$  Index) and to (*d*) the kurtosis (AMAk Index) of gravity changes over the whole domain

Figure 9. Spatial distribution of the mean and variance of drawdowns and of total Sobol' indices associated with  $S_s$ ,  $S_Y$ ,  $a_{kD}$ ,  $K_D$  calculated throughout a vertical cross-section at times t = 100 s, 4 hours, and 7 days.

Figure 10. Spatial distribution of the mean and variance of gravity changes and of total Sobol' indices associated with  $S_s$ ,  $S_y$  and  $a_{cD}$  calculated throughout a vertical cross-section at times t = 100 s, 4 hours, and 7 days.



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