

# Elementary District Metered Areas design of looped water distribution networks with multiple sources

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**Abstract:** the identification of district metered areas (DMAs) in existing water distribution networks is a complex problem, whose solution strongly depends on the type of network. The purpose of the present work is to present a new methodology to identify isolated DMAs in water distribution network having a large number of water sources, such as autonomous reservoirs (for example supplied by pumping stations) directly connected to the distribution network. The method proposed is based on graph theory. Initially, the network is subdivided to the least terms into “elementary” districts, areas of influence of each supply source. In a second step, the resulting elementary areas are conveniently united to form possible layouts of DMAs solutions, respecting the criteria of independency, isolation and self-sufficiency. The union process takes into account the size of districts and the overall network resilience. The

23 method is effectively applied to a case of study and the resulting DMA layouts of the network  
24 can be used to compare different preliminary solutions of large network partitioning.

25 **Author keywords:** water distribution networks; multi-sources; district metered areas; graph  
26 theory; isolated districts; elementary districts.

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28

## 29 **INTRODUCTION**

30 The design of district metering areas in existing water distribution networks (WDNs) represents  
31 a complex problem, as often water networks have been designed in successive stages and not  
32 conceived with a district metering perspective. However in the last decade this techniques has  
33 been used to improve water distribution system management and efficiency.

34 The redesign consists in partitioning a WDN into permanent areas, called District  
35 Metered Areas (DMAs), in which flow quantities entering and leaving are metered. Two  
36 categories of district metering design can be defined. The first one aims to partition the network  
37 into permanent district metered areas (DMAs), by means of the insertion of boundary valves;  
38 the second one is tailored for multiple source networks and consists in dividing the original  
39 network into isolated sectors self-sufficient and independent in terms of water supply (isolated  
40 DMAs). This latter category, whenever possible, is even more effective in terms of providing  
41 water protection from accidental or intentional contaminations (Di Nardo et al. 2015).

42 The principal aims of superimposing DMAs to a looped or branched system are: to  
43 simplify water balance calculation with flow meter strategically placed on every zone,  
44 enhancing in this way the control on water losses and thus the usage of water and energy  
45 resources; to limit the effect of a potential contamination by partitioning the network into  
46 smaller areas; to manage pressure more efficiently on every zone, saving energy and

47 maintenance costs. The continuous monitoring allows an overall more efficient management  
48 of the total system behaviour.

49         Several studies and technical reports containing general guidelines for dividing a water  
50 distribution network into district metered areas are available in literature (Farley, 1985; WRc  
51 1994; Butler, 2000; Morrison et al. 2007; Baker, 2009). The essential factors to take into  
52 account are: the size limits of the DMAs, recommended generally between 1,000 and 3,000  
53 users, accordingly to WRc guidelines, or between 2,500 and 12,500 inhabitants (Butler 2000);  
54 the connectivity properties (as a result fire flow and pipe bursts conditions, but also water age  
55 could be affected by DMAs); and pressure constraints at demand nodes. Other factors can be  
56 final leakage level target, implementation and maintenance costs.

57         For a given WDN, a number of possible DMAs layouts exists and the definition of the  
58 final solution, in order to not lower the system performance, should rely on a variety of  
59 conflicting objectives, such as cost, reliability and water quality. Many are the methodologies  
60 proposed in literature with the aim to redesign water supply distribution network and  
61 considerable work has been done to improve and automate the design, planning and  
62 management of DMAs.

63         Due to problem complexity, traditional methods were mainly based on a trial and error  
64 approach over the network hydraulic and water quality models to design a WDN into sectors,  
65 (Charalambous, 2005; Macdonald and Yates, 2005; Rogers, 2005). After an initial division into  
66 districts the solution was evaluated by performing hydraulic simulations under different  
67 demand scenarios (average demand, maximum demand, fire flow, pipe burst). The final  
68 solution was obtained by manually modifying the water network until the design constraints  
69 were met, i.e. an acceptable solution found.

70         Graph theory (Jacobs and Goulter, 1989) has been proved an useful tool for modelling  
71 water distribution network, thus also when solving DMA design problems (Ostfeld and Shamir,

72 1996; Ostfeld, 2005; Tzatchkov et al. 2006; Sempewo et al. 2008, Herrera 2011). By means of  
73 graph principles the structure of the network can be simplified and the complexity of the  
74 problems is likewise reduced. The properties of connectivity among nodes within a DMA and  
75 reachability of nodes from the water supply sources are extremely important when redesign a  
76 water distribution network into DMAs.

77         Considering size constraints and connectivity properties, Ferrari et al. (2013) recently  
78 developed an automatic methodology, based on a recursive bisection algorithm, which  
79 determines DMA boundaries so that districts are sized between imposed limits, independent  
80 one from another and compliant to minimum pressure requirements.

81         Another aspect to address is the optimal positioning for boundary valves and flowmeter.  
82 In this regard, Di Nardo and Di Natale (2010) developed a decision support system for DMA  
83 design using some graph theory approach to identify minimum dissipating power paths. By  
84 computing a frequency value for every pipe, proportional to the number of times that a given  
85 pipe is found in one of the aforementioned paths, flowmeters are candidate on higher frequency  
86 pipes, while boundary valves are inserted on lower frequency pipes.

87         Using similar principles, Alvisi and Franchini (2014) proposed an automatic procedure  
88 to generate different districts layouts, for an assigned number of DMAs, i.e. meters to be  
89 installed. Firstly, without performing a hydraulic simulation, possible solutions are found with  
90 the aid of the Breadth-First Search (BFS) algorithm: network nodes are grouped in sets,  
91 according to their weighted distance from a source node, proportional to head loss. Districts  
92 are then defined as groups of node sets having total water demand within the recommended  
93 limits. Several possible flowmeters and closure valves locations are then compared, now  
94 performing a hydraulic simulation. It is selected the layout that maximises the system  
95 resilience.

96

97

98           In general, it is preferable to consider more DMA layouts in the way to support a more  
99 flexible decision process. The best solution is always identified by a comparison, which  
100 depending on the problem goals and constraints can be based for example on network resilience  
101 (Prasad et al 2004; Alvisi and Franchini 2014), pressure and flow deficit indices (Giustolisi et  
102 al. 2008b).

103           Whereas several design criteria and constraints are considered, graph theory approach  
104 is often coupled with evolutionary algorithms allowing to evaluate several DMA possibilities  
105 (Hajebi et al. 2014; De Paola et al. 2014, Giustolisi and Ridolfi 2014). However the needs of  
106 planners are sometimes difficult to traduce in mathematical terms and thus optimization  
107 methods based on the definition of a precise multi-objective function cannot be easily applied  
108 (Di Nardo et al. 2010).

109           A significant role on the choice of the technique to adopt is played by the network size  
110 and its characteristics. In particular whenever the number of sources is elevated it can be  
111 preferable to create isolated districts, each fed by a single or a set of independent sources.

112           In this regard, Di Nardo et al (2013) used graph theory to recognize minimum  
113 dissipating power connecting each source to nodes, and thus using a GA to swap the nodes and  
114 subdivide the network into isolated DMAs. Some other graph theory approaches of WDN-sub-  
115 zoning problem, are described and compared by Perelman et al. (2014), among which graph  
116 partitioning and community structures. In particular, the community structure concept, which  
117 is a property in common to many of complex systems, is used by Diao et al. (2013) to  
118 decompose the network into a layout sub-systems which are seen as basis for DMA planning.  
119 However not every distribution network has been developed from the union of communities,  
120 thus when dealing with very large water distribution networks densely looped and supplied by

121 multiple sources, developing an automatic procedure to decompose the original network into  
122 sub-systems with the required properties can require a more generic technique.

123 Sometimes it can be undesirable to subdivide very large systems into DMAs of  
124 recommended size in one single step. Because most of them were originally conceived as a  
125 single unity, and are still working as such. Moreover for networks fed by an elevated number  
126 of sources an eventual partitioning of the network can result in a sensible change in water use  
127 for the different sources.

128 To tackle all these aspects, this study exploits the concept of “influence area” of a  
129 supply station. This concept, already proposed in the design phase to build up large networks  
130 (Swamsee et al. 2008), can be reversed and used to decompose an existing network into  
131 isolated subsystems with independent input sources.

132 An automatic procedure is here proposed for subdividing into DMAs a large WDN  
133 supplied by several sources. The method is based on graph theory principles and a criterion of  
134 prevalent supply contribute by the different sources. In particular the design criteria adopted  
135 are: direct access to water sources; to limit the DMA effects on water use, i.e. changes between  
136 sources; to respect of minimum pressure on nodes requirement.

137 After a preliminary analysis and hydraulic simulation, the network is mapped as an  
138 oriented graph, using weights proportional to pipe flows, every node of the network is assigned  
139 to its most important water source in terms of demand contribute. As a result the original system  
140 is subdivided into as many influence areas as the number of sources feeding the network. Each  
141 area representing a portion of the network predominantly supplied by a given source, so that  
142 boundaries closure will not deeply affect the original water use of sources.

143 In particular influence areas can be then considered as isolated districts, when  
144 boundaries are closed, or as mere “functional” districts when not confined by a physical border.

145 Hereafter they will be referred to as “elementary” districts (eDMAs), for they represent the  
146 least possible subdivision of the network into independent and isolated areas.

147 Once identified, elementary districts can be used as bricks with which compose  
148 different DMA layouts. The coupling procedure developed is based on resilience  
149 maximization. It is fast and flexible procedure, able to adapt to different objectives and to the  
150 level of sectorization the water utility is interested to, leaving open the possibility for a  
151 progressive sectorization.

152 Finally the method effectiveness is tested on large WDN, case study derived as a portion  
153 of a real distribution system. Some consideration on the possible application of the method are  
154 drawn in the conclusion.

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156

## 157 **METHODOLOGY**

158 The proposed methodology consists of three main steps:

- 159 1. Preliminary analysis of the network, in order to identify main supply sources, that are those  
160 regularly active at peak demand conditions;
- 161 2. Elementary DMAs (eDMAs) creation by means of graph theory based algorithms. A  
162 number of district equals to the number of sources is generated: each source will have its  
163 own eDMA and each eDMA is supplied only by one source;
- 164 3. Progressive eDMAs union according to size constrains and to a criterion of resilience  
165 maximization with **an automatic iterative** procedure. During the union procedure the  
166 physical borders of eDMAs are removed to make the fusion itself possible, and they are  
167 close again once the optimal union has been selected, **from a functional point of view**.

168

### 169 **Input Data**

170 The proposed methodology requires the following input data:

- 171 • The hydraulic model of the water distribution network, nodal base demand, demand  
172 patterns;
- 173 • Minimum required pressure over nodes, in the way to verify the feasibility of the resulting  
174 solutions;
- 175 • A criterion to drive the eDMA union process. In this work a maximum resilience rule is  
176 selected.

177

178

### 179 **PRELIMINARY ANALYSIS**

180 A preliminary analysis of the original network configuration is carried out to acquire some  
181 topological information, among which is of particular interest the number of source nodes ( $N_S$ )  
182 present in the distribution system, as this information will be essential in next phase when an  
183 equal number of elementary DMAs has to be identified.

184 Subsequently, a complete hydraulic simulation of a demand driven model of the  
185 network is performed through EPANET 2.0 (Rossman, 2000) and pipes flows at peak hour are  
186 evaluated. Water losses were not explicitly considered but evenly distributed, included in the  
187 estimated nodal demands.

188 Pipes flow data are used to draw two matrices. Adjacency Matrix (AM), containing all the  
189 connections between the network nodes, is defined as:

$$\begin{cases} AM(i, j) = AM(j, i) = 1 & \text{if exists a link between nodes } i \text{ and } j \\ AM(i, j) = AM(j, i) = 0 & \text{otherwise} \end{cases} \quad (\text{Eq. 1})$$

190 Flow-weighted adjacency Matrix (FM), which considers the flow directions through the weight  
191  $w_{ij}$  defined as:



$$w_{ij} = \frac{1}{|q_{ij}|} \quad (\text{Eq. 2})$$

192 where the discharge flowing from node  $i$  to node  $j$  is called  $q_{ij}$  and it is a positive value if the  
 193 flow direction is from node  $i$  to  $j$ , negative otherwise.

194 The FM compilation, where FM is not a symmetric matrix, is carried out following the  
 195 subsequent law:

$$\begin{cases} FM(i, j) = w_{ij} & \text{and} & FM(j, i) = 0 & , \text{if } q_{ij} > 0 \\ FM(i, j) = 0 & \text{and} & FM(j, i) = w_{ij} & , \text{if } q_{ij} < 0 \\ FM(i, j) = FM(j, i) = 0 & & & , \text{if } q_{ij} = 0 \end{cases} \quad (\text{Eq. 3})$$

196

197

## 198 **ELEMENTARY DISTRICTS CREATION**

199 A number  $N_s$  of elementary districts is created, equal to the number of sources identified during  
 200 the preliminary analysis. Extension of these "functional" eDMAs obviously varies in time with  
 201 water demand. However, in most cases variation is referred to a limited zone along the border  
 202 in common between different influence areas, where network nodes are supplied quite equally  
 203 by two or more sources. For this reason in the presented method, a reference demand condition  
 204 is considered for eDMAs identification, corresponding to the maximum peak demand.

205 In order to define each eDMA domain, the network is explored using graph theory  
 206 algorithms and the FM matrix just created, in order to identify which nodes are belonging to  
 207 which sources. In particular two algorithms will be used, Breadth First Search (BFS) and  
 208 Dijkstra algorithm.

209 Firstly, following flows direction from a given source node  $S_j$ , the BFS algorithm  
 210 allows to obtain a nodes list  $NL$  of all nodes supplied by the source. A  $NL$  is thus created for  
 211 every source  $S_j$  of the network  $NL(S_j)$ . If a generic node  $x$  is a downstream node of source  $S_j$ ,  
 212 this means that the water delivered from node  $x$  derives totally or partially from that source.

213 Due to the looped nature of the network, generally few nodes are reached only by a single  
214 source, more frequently, a node is reached from several sources. In the former case, the node  
215 can be immediately assigned to the eDMA built around its single source; in the latter and more  
216 frequent case, instead, the choice of the eDMA assignation is less obvious and it will be carried  
217 out with additional steps. Looking at the example in Fig. 1, the node list of sources  $S1$  and  $S2$   
218 are  $NL(S1) = \{a, b, c, e, f, g, h, m, p\}$  and  $NL(S2) = \{a, c, d, f, h, p\}$ .

219 Similarly also a sources list ( $SL$ ) for each node can be created, i.e. a list of every source  
220 a given node depends on. It can be observed that nodes  $b, e, g$  and  $m$  are reached only from  $S1$ ,  
221 while no flow contribute from  $S2$  arrives to them, so they directly belong to the influence area  
222 of the former source, hereafter called eDMA-1, see Fig. 1. Similarly node  $d$  belongs to eDMA-  
223 2. For all other nodes, reached by more than one source, such as nodes  $\{a, c, f, h, p\}$ , a univocal  
224 assignation rule has to be established: a node  $x$  belongs to eDMA- $j$  if the most of water  
225 delivered from node  $x$  derives from source  $S_j$ . In other words, this means that the principal  
226 source of that node  $x$  is the DMA  $j^{\text{th}}$ -source. In practice this result can be achieved through two  
227 additional steps: Dijkstra's algorithm (Dijkstra, 1959) and main path identification.

228 Dijkstra's algorithm is exploited in order to find out the main paths (i.e. the shortest)  
229 connecting node  $x$  to its supplying sources, using the weighted adjacent matrix  $FM$ , see  
230 different grey shades for main path in the example of Fig. 1. Since  $FM$  weights are inversely  
231 proportional to the pipe flow (see Eq. 2), the shortest path resulting from the algorithm is the  
232 one made up of the pipes conveying the largest flow that connect the given node to a source.  
233 The main paths connecting a given node to its multiple sources may have a common part, i.e.  
234 waters from the sources mix up before reaching the node. This is the case of nodes  $\{a, c, f, h,$   
235  $p\}$  in Fig. 1.

236 In any case to assign a node to one of its supplying sources, it is necessary to compare  
237 the discharge contributes coming from the different sources. At every fork on the main paths,

238 automatically tracing backward the flows from the given node, it is followed the direction from  
239 which comes the highest flow. In this way the prevalent supplying source is found. For  
240 example, considering node  $a$  in Fig. 3 there are two main paths,  $p1=\{S1, b, c, a\}$  and  $p2=\{S2,$   
241  $d, c, a\}$ , and the common path is  $\{c, a\}$ . So in order to discriminate the main source for node  $a$ ,  
242 a comparison is made between the flows of the pipes immediately upstream the fork, pipe  $bc$   
243 for path  $p1$  and pipe  $dc$  for path  $p2$ , and the highest one determines to which source's influence  
244 areas assign node  $a$ . Since  $q_{dc} > q_{bc}$ , then  $eDMA(a) = S2$ . If two directions are equal in flow,  
245 then, always on the mains paths, this time the comparison is made between the flows of the  
246 pipes immediately upstream the ones previously considered.

247 This procedure, reported also in the flow chart of Fig. 2, has to be repeated for every  
248 node of the network leading to a subdivision into groups of nodes. Groups are referred to a  
249 specific source, gathering its node domain. This means that every group forms an eDMA.  
250 However a group of nodes has not been addressed by the procedure yet: it is the set of no-  
251 demand nodes, connected to the network by non-active pipes. The nodes of this group and other  
252 assignation errors will be assigned by the iterative refining procedure exposed in next  
253 paragraph.

254

### 255 **Iterative Refining Procedure**

256 The last step of the elementary district creation procedure aims to refine the node assignation,  
257 in order to consider also those nodes lying on non-active links, i.e. links not conveying flow.  
258 In addition, the effective continuity of each elementary district domain has to be verified. It  
259 may happen that some nodes belonging to a group are not directly connected each other, but  
260 spaced out by node belonging to a different set. After the closure of pipes in order to create the  
261 district boundaries, these nodes will result isolated. Therefore, network nodes are categorized  
262 into two different sets: biological and foster nodes. To this aim it is useful introducing a

263 definition of “parent” nodes: for every network nodes its “parents” (note they could easily be  
264 more than two) are its immediately **upstream nodes** (considering every upstream flow  
265 direction). Furthermore, this analysis is carried out at peak demand conditions, so the flow  
266 directions are not changing. In this way it is possible to establish that all nodes having at least  
267 one parent belonging to their same eDMA are called biological; vice versa a generic node  $x$  is  
268 considered foster if one or more of the following definition are met:

- 269 • If the node  $x$  has been previously assigned to an eDMA different respect to all of the  
270 assignments of its immediately upstream nodes;
- 271 • If  $x$  has not been assigned yet to any particular eDMA;
- 272 • If  $x$  is a node on a non-active pipe and all immediately adjacent nodes has been assigned  
273 to eDMAs different than  $eDMA(x)$ .

274 Considering Fig. 3, node  $h$  initially assigned to Source  $S2$  is a foster node because its  
275 parents (nodes  $f$  and  $g$ ) are both belonging to  $S1$ . In the same way also node  $q$  is a foster child,  
276 since it is a no-demand node and no discharge is flowing in pipe  $dq$ . So, during  $FM$  creation,  
277  $FM(d,q)=FM(q,d)=0$  and then the procedure stopped without assigning an eDMA to node  $q$ ,  
278 because its source list is empty ( $SL(q)=\emptyset$ ). Node  $q$  is then considered as foster and needs to be  
279 assigned to a proper eDMA.

280 On contrary, node  $p$  is not considered at first a foster node, even if it is a grey node in the  
281 middle of black ones. Accordingly to the definition above-mentioned, there is at least one of  
282 its parents (in this case corresponding to node  $h$ ) belonging to its same eDMA. It will be  
283 addressed for correction in a subsequent iteration step.

284 In fact, the refining method proposed is an iterative and recursive procedure that, at  
285 every loop, controls all network nodes and if recognizes foster nodes assigns them to their  
286 proper eDMA. Since the  $FM$  matrix contains only the nodes connections where the discharge

287 is not null, the refining procedure employs also the  $AM$  matrix, which contains all the links  
288 present in the network, independently on flows.

289 The correction rules implemented for foster nodes founds at every iteration are the following:

- 290 • If node  $x$  is linked to the rest of the network by one or more pipes with  $q \neq 0$ , node  $x$  will  
291 be assigned to the same eDMA of the parents from whom link derives the maximum flow  
292 (see node  $h$  in figure 3 and 4);
- 293 • If node  $x$  is located on a no flow pipe ( $q=0$ ) and all of the nodes connected (by links) to  $x$   
294 belong to a unique eDMA, then node  $x$  will be assigned to the same eDMA of the  
295 connected nodes (this is the case of node  $q$  in fig. 3 and 4);
- 296 • Otherwise the generic node  $x$  cannot be assigned yet, it will be reassigned in the following  
297 loops.

298 During the iterations it may happen that new foster nodes are discovered or already assigned  
299 nodes turn foster again. The procedure of searching and refining stops when no more foster  
300 nodes are left.

301 As a result of the assignation procedure every source corresponds a more or less  
302 extended influence area. The elementary districts creation is completed by defining the  
303 boundary of each district, which means that in the hydraulic model all pipes connecting nodes  
304 belonging to different district are closed (see Fig. 5). In the particular case of WDN supplied  
305 by pumping stations having modified network asset, the creation of eDMAs could be coupled  
306 with a revision of the pump schedule in order to maximize the partitioning benefits also in  
307 terms of energy consumption. However elementary areas can be just a necessary but  
308 intermediate step of the proposed DMAs procedure, for this reason the feasibility and respect  
309 of every given constraint will be fully addressed in the following union procedure.

310

311

312 **PROGRESSIVE eDMA UNION**

313 Once the original network is subdivided into eDMAs, it is possible to assemble them as if they  
314 were bricks with which build up different macro isolated DMA layouts (mDMAs).

315 Generally, it is unlikely and inconvenient to have a large number of DMAs. Even if it  
316 implies a greater network control, it is unfeasible from an economical and practical point of  
317 view since it would require the closure of a large number of pipes and to install valves between  
318 DMAs.

319 Therefore elementary districts in the following steps are conveniently coupled to the  
320 respective neighbouring districts, in order to form bigger isolated districts. A complete  
321 coupling method requires the evaluation of all possible combinations of joined districts in a set  
322 of two, three, four, etc., in order to find out the optimal grouping layout. However, for  
323 multisource networks, the number of possible combination reaches very high values even with  
324 low numbers of eDMAs and computational times becomes unfeasible. Therefore, it is essential  
325 to reduce this number and this is carried out through a progressive union procedure driven by  
326 a performance criteria.

327 In fact, by sectorizing a WDN, the vulnerability of the network increases because  
328 independent districts cannot anymore rely on nearby sources to overcome emergency  
329 situations, (Grayman et al. 2009). Thus the union procedure should select most performing  
330 unions from resilience point of view: the performance criteria is based on Todini's resilience  
331 index (Todini, 2000), which is a good measure of system capability to overcome failures and  
332 unexpected water demands (i.e. pipe bursts and fire flow demands).

333 Todini associates the capability of the system, to react overcoming stress conditions,  
334 with the energetic redundancy. According to him, the resilience index  $I_r$  of a water distribution  
335 system, after appropriate substitutions, can be written as:

$$I_r = \frac{\sum_{i=1}^{N_n} d_i (h_i - h_i^*)}{\sum_{k=1}^{N_s} Q_k H_k - \sum_{i=1}^{N_n} d_i h_i^*} \quad \text{Eq. 4}$$

336 where  $d_i$  is the nodal demand and  $h_i$  the hydraulic head at each node  $i$ , with  $N_n$  the total number  
 337 of network nodes;  $Q_k$  and  $H_k$  are the discharges and head at the  $N_s$  source nodes;  $h_i^*$  is the  
 338 minimum pressure head.

339 These are the main steps of the union procedure:

- 340 1. at first, the procedure individuates the “small” elementary districts, whose size is below or  
 341 equal to  $N_0$  (which could be for instance the smallest size of the obtained eDMAs);
- 342 2. each small district is tentatively united to its neighbouring ones, by temporary re-opening  
 343 pipes on common boundaries; each different union possibility is compared to the others by  
 344 looking at the respective Todini’s index. For each small district, among all the union  
 345 possibilities, the one which maximize the resilience index and at the same time respects the  
 346 minimum pressure criterion is selected;
- 347 3. the districts selected for union are joined together by definitively opening the boundaries  
 348 they have in common. These latter and the other unchanged districts form a new DMA  
 349 layout;
- 350 4. the threshold for minimum number of nodes can be further increased by  $\Delta N$  (a reference  
 351 value could be obtained as the difference between the largest and smallest size of eDMAs  
 352 divided by twice the number of internal sources), and the procedure starts back from point  
 353 1, creating a different district layout starting from the one obtained on point 3 during the  
 354 last cycle.

355 In the present paper, the size of districts is evaluated accounting the number of customers  
 356 supplied within the DMA. The district union procedure proposed is also summarized in the  
 357 flow chart of Fig. 6 and in the drawing of Fig. 7. The algorithm stops either when the original

358 network is sectorized into a satisfactory number of mDMAs or when only 1 mDMA is left,  
359 scenario of course coinciding with the original network.

360 In this work, Scenario-1 is called the first scenario created with the elementary DMA creation  
361 procedure (Fig. 7a), which corresponds to the maximum sectorization of the network; scenario-  
362 N, the last scenario resulting from the union procedure (Fig. 7f), with only one mDMA which  
363 corresponds to the entire network.

364

365

## 366 **OUTPUTS**

367 The proposed procedure allows for: the division of an existing network into elementary districts  
368 (eDMAs), minimum size subsets of the network; and the successive creation of different  
369 mDMA scenarios by progressively unifying eDMAs. At every step of the method it is verified,  
370 performing a hydraulic simulation, that during the whole simulation period each node is  
371 associated with a pressure equal to or greater than the minimum required. If this constraint is  
372 not met, then the solution is discarded as unfeasible in favour of other eDMAs unions.

373 Several possible DMA scenarios are achieved for the water distribution network under  
374 examination, different for districts number and district size, depending on the imposed size  
375 threshold. These layouts should be seen as a preliminary subdivision of the network, leaving  
376 to a second phase the definition of optimal boundaries. For every mDMA scenario is always  
377 checked that minimum size and internal connectedness requirements are met.

378 Performance indices (PIs) are used to compare the new layouts of the sectorized water  
379 distribution system and the original system. In particular three PIs were computed to assess the  
380 alterations in the hydraulic behaviour due to partitioning:

- 381 a. Resilience index: Todini (2000) proposed a resilience index to give an idea of the ability of  
382 the network to supply water demands even under stress conditions, as associated with



383 energetic surplus in the system which can be beneficially dissipated inside the network in  
384 case of mechanical (pipe bursts, pumps out of service, power outages, etc...) or hydraulic  
385 failures (unexpected changes in demands or pressure heads).

386 b. Entropy index: Awumah et al (1991) defined a PI directly related to the loop degree of the  
387 system, i.e. multiple flow paths from the water source to the demand nodes. In case of  
388 failures, if a certain pipe is interrupted, the nodes can be supplied through another path,  
389 allowing the maintenance of the service to the customers. Because of the subdivision into  
390 zones, it is important to control that this ability is not sensibly diminished.

391 c. Hydraulic Performance pressure related (HP): introduced by Coelho in 1997, it is based on  
392 the assertion that, for a network to perform well from a hydraulic point of view, the pressure  
393 at every supply point must range between a maximum and a minimum requirement. In  
394 particular, a network HP is high if nodal pressures is close to the lower pressure bound, as  
395 this means that there is less energy waste while meeting the prescribed demands.

396 These PIs will also help in later decision process to select the final “best” solution of DMA  
397 layout among the ones obtained. To be thorough, also water quality and the ability of the  
398 resulting system to meet fire flow conditions should be checked. In fact, the creation of DMAs  
399 has been proven to affect the capability of meeting fire flow conditions as well as the reliability  
400 and the quality of the water delivered.

401 In general, the “best” solution will be identified only after all relevant levels in decision  
402 process have been involved, taking into account for instance also non-functional, such as water  
403 quality issues, administrative patrols, homogeneity of user typologies. Nonetheless, the  
404 proposed method could be considered a helpful tool at preliminary stage where it is important  
405 to analyse and compare different possible DMA solutions.

406

## 407 **CASE STUDY**

408 The methodology presented in this work is expressly thought for multi-sources, highly  
 409 looped water distribution networks. Effectiveness of the method is thus tested with the  
 410 application to a large WDN case study. It is a portion of a larger real network, in which the  
 411 number and position of supply sources was slightly modified (Fig. 10).

412 The total length of the water distribution network is about 506 km, reaching more than  
 413 330,000 end users and with total demand coverage. The web-like network follows the city's  
 414 street layout and make it a highly looped system. The network is supplied by 7 reservoirs, of  
 415 constant head, sparse around the network, called Res. A, B, ..., G, from which are named the  
 416 districts. The proposed method has been implemented in MATLAB Release 2010a. The  
 417 hydraulic analysis has been performed by using the simulation software EPANET (Rossman,  
 418 2000). The head losses are evaluated with the Hazen-Williams formula. Other basic  
 419 characteristics are reported in Table 1.

420 The minimum number of customers per district  $N_{threshold}$ , defined in Eq. 5 and initially  
 421 set equal to  $N_0$ , can be considered dependent on the case study considered. For this reason, in  
 422 the present study, the size threshold has been made dependent on the minimum and maximum  
 423 eDMA sizes obtained (namely  $N_{min}$  and  $N_{max}$ , see Eq. 6) and it will be incremented by the  
 424 quantity  $\Delta N$ , Eq. 7, in the way that different scenarios of network partitioning are obtained.

$$N_{threshold,i} = N_0 + (i - 1) \cdot \Delta N \quad \text{Eq. 5}$$

$$N_0 = N_{max} - N_{min} \quad \text{Eq. 6}$$

$$\Delta N = (N_{max} - N_{min}) / (2 \cdot N_s) \quad \text{Eq. 7}$$

425 where  $N_s$  is the number of internal water sources (7 in this study, as the number of reservoirs);  
 426  $N_{threshold}$ ,  $N_0$ ,  $N_{min}$  and  $N_{max}$  are expressed in number of equivalent costumers.

427 In order to better show how the presented method works, in this application no maximum size  
428 has been set, thus the union process will continue until the last DMA scenario coincides with  
429 the entire distribution network.

430

## 431 **Results**

432 According to the proposed method, at first isolated elementary districts are identified  
433 inside the original water distribution. This preliminary sectorization is achieved exploring the  
434 network as a graph, in particular using Breadth First Search and Dijkstra algorithms.

435 By means of BFS algorithm only those nodes relying on a single water source can be  
436 assigned, so at the first step just a preliminary node assignment to one of the 7 sources is  
437 achieved: 2,801 nodes, about 54% of the nodes, are assigned. Successively, by means of the  
438 second and third steps, main paths are identified (Dijkstra's algorithm) and compared in terms  
439 of discharges along them, making thus possible to achieve the assignment of 4,971 nodes,  
440 which is more than 95% of the whole network. Finally, the rest of the nodes is assigned by  
441 means of the refining procedure, which not only assigns the nodes that were not in the previous  
442 steps (i.e. all the nodes relying on non-active links) but also verifies and eventually corrects  
443 already assigned nodes, in the way that all the elementary districts consist in an interconnected  
444 set of nodes supplied by at least one of the internal sources of the network.

445 As a result of the creation of elementary districts, the original network is subdivided  
446 into 7 eDMAs, corresponding to the 7 reservoirs. This eDMA layout, called Scenario-1, is  
447 represented in Fig. 9.

448 Follows the elementary districts union procedure, which is driven by a progressively  
449 increasing supplied-customers threshold. Starting from the eDMAs scenario (Scenario-1), a  
450 second layout of districts is obtained by coupling small eDMAs with the neighbouring ones,  
451 the initial threshold is  $N_{threshold,1} = 24,480$  customers/district (Eq. 5, 6). The second layout

452 of districts is thus called Scenario-2, and it is composed only by mDMAs of size major than  
453  $N_{threshold,1}$ . It is the results of a recursive procedure: every possible union of small districts  
454 with adjacent districts is evaluated, and the best union is selected by a comparison, of feasible  
455 solutions (i.e. respecting the minimum pressure requirement), based on resilience index  
456 (Todini, 2000). Subsequently new mDMAs layouts are obtained by increasing progressively  
457 the threshold size,  $\Delta N$  by  $\Delta N$  (where  $\Delta N = 3,587$ , Eq. 7). At each cycle, only the unions  
458 leading to the highest improvement of the network are actuated, giving birth from Scenario-1  
459 to 6 more district scenarios, represented in Fig. 10. Note that the Scenario-7 corresponds to the  
460 original network, and consider that the number of obtained layouts and the number of reservoirs  
461 coincides just by chance, and it is not a result sought by the method.

462 The union process history can be reconstructed following the line paths in Fig. 11. Each  
463 grey column represents a scenario, from 1 to 7, corresponding to the network layouts of Fig.  
464 10. For every district (A, B, ..., G) is reported, on the left side of the chart, the number of  
465 customers in each eDMAs (Scenario-1). The number of DMAs per scenario is equal to the  
466 number of horizontal lines crossing each column. In the same way the number of customers  
467 per DMA in a scenario can be calculated summing the initial numbers of connections at the  
468 beginning of the corresponding line paths.

469 The resulting characteristics of the DMA layouts are summarised in Table 2. Each row  
470 is a scenario, while in columns are reported: the numbers of DMAs for the layouts; three  
471 performance indexes, resilience, entropy and hydraulic performance pressure related; and lastly  
472 there are 7 more columns reporting the daily total water demand supplied by the seven  
473 reservoirs.

474 As expected, resilience index decreases with the number of DMAs. However, with the  
475 proposed approach resilience seems to be not so much affected by sectorization. It has to be  
476 noted that resilience values are very high in all scenarios, near unity, due to the significant

477 energy level in the original system. Entropy values, which reflect the densely interconnected  
478 nature of the network seems to remain stable, despite the reduction of possible flow paths. So  
479 does hydraulic performance pressure related.

480 In conclusion, the results show only slight changes for all the three performance indicators  
481 respect to their values in the original network (last row, Scenario-7). This can be explained by  
482 three considerations. First, the eDMAs identified with the proposed approach are essentially  
483 already present in the original network from a functional point of view as influence areas of  
484 supply sources. Second, the number of reservoirs distributed uniformly enough over the  
485 network guarantees a high level of energy in normal conditions. Third, the different mDMAs  
486 layouts are selected according to maximum resilience criterion and the respect of minimum  
487 nodal pressure requirement.

488 In Table 2, it is also reported the total water volume (expressed in  $10^4 \text{ m}^3/\text{day}$ ) supplied by each  
489 reservoir which varies in the different considered layouts. While some reservoirs show  
490 negligible changes in their daily consumptions, as in the case of reservoir C, it is not so for  
491 some others, which undergo a small increment of water demand, especially in Scenario-1. For  
492 example, reservoir G undergoes an increment of daily demand about +10% in the more  
493 sectorized scenarios (Scenario-1 and 2), which could be considered a feasible change. The  
494 storage supplying capacity and the feasibility of these changes should be checked and it plays  
495 a role in the successive decision phase over the most convenient scenario to adopt, both  
496 considering water availability and water quality of the different sources.

497 Another important checks to carry on for the obtained DMA layout, in order to test the  
498 goodness of each solution, could be to test fire flow conditions and water quality.

499 The first control aims to test the ability of the different DMAs layouts to respond to fire flow  
500 emergency in compliance to the minimum pressure constraint. This test should be carried out  
501 at the most critical situation, therefore at maximum daily demand condition, a fire flow has

502 been superimposed. The fire flow demand has been assumed of 15-30-60 lps, where 30 lps is  
503 in line with Italian typical design standards. For each district, an interior node particularly  
504 disadvantaged for pressure and located on pipe of small diameter ( $\leq 150$  mm) has been selected.  
505 The nodes selected are also located on the new border of each district, as they are most  
506 significantly affected by the closure of boundaries between districts. Although there is no  
507 guarantee that the selected node would be the most critical, this can be still considered a  
508 reasonable and short way to test the ability of the system to respond adequately to a fire  
509 condition (Grayman et al. 2009). The fire flow condition was tested by sequentially and  
510 separately applying the fire flow demand to the selected nodes, and the result is always positive  
511 for all nodes in case of a demand of 15 lps. For the selected nodes, in the original network, the  
512 fire flow control was always positive with pressures at all nodes over 20 meters. When a higher  
513 fire flow demand has been tested, such as 30 lps and 60 lps the analysis showed that the pass  
514 rate decreases respectively to 75% and 33 % of the DMAs. Overall the fire flow control is  
515 positive, especially considering that the tested disadvantaged nodes are on DMA boundaries,  
516 which have not been optimized yet, being beyond the aims of the presented work, therefore a  
517 better arrangement of border can definitively help the respect of fire flow condition also for  
518 this few nodes.

519 The last control over DMA scenarios is water quality, specifically water age, which can  
520 be considered as an information of the potential water quality deterioration in the network. It  
521 is the parameter controlled to compare the performance of the network in the original asset and  
522 in the DMAs layouts. Both water age and demand-weighted average water age are calculated.  
523 The results, reported in **Table 3**, show that there is no significant difference between the average  
524 water ages of the whole system in the alternative designs in comparison to the original values.  
525 Of course there can be a noticeable difference in water age node by node, but this does not  
526 affect remarkably the overall behaviour of the network, (Grayman et al. 2009).

527

528

529 **CONCLUSION**

530 A new methodology for designing isolated DMAs for highly looped multi-source WDNs is  
531 proposed by the authors. The method is based on graph theory: it applies graph theory  
532 principles and algorithms to determine the least isolated districts subdivision of the existing  
533 network. These districts are called “elementary” districts, eDMAs, and should be seen as  
534 influence areas of each water source present in the distribution network.

535 Subsequently the eDMAs are progressively united each other by a union procedure,  
536 driven by a maximum resilience criteria and accordingly to their size, obtaining thus different  
537 isolated macro-DMA scenario levels of network sectorization. ~~The connectivity properties of~~  
538 ~~the district nodes with the water source are considered in the process of creating the eDMAs,~~  
539 ~~thus also subsequent mDMAs scenarios.~~

540 The proposed methodology is specifically tailored for large distribution network  
541 supplied by a high number of sources, a complex case not usually treated in literature. Here it  
542 is hard to decide a priori a number of districts in which subdivide the network, because of its  
543 high redundancy and the distribution of the internal supply stations. For this complex systems  
544 the method presented is able to identify the influence areas of each internal source and then  
545 building different levels of DMAs scenarios without changing the natural dependence of nodes  
546 on supply stations. The achieved layouts are all made by self-sufficient and isolated DMAs,  
547 without a sensible decrease of the overall performance of the network, in terms of resilience.

548 Every scenario, every union decision, is obtained by performing hydraulic simulation in order  
549 to verify the compliance of the minimum pressure requirements. Moreover, being originated  
550 from influence areas, each DMA scenario does not affect significantly the previous water uses  
551 coming from the different sources. The resulting sectorization solutions of the network enable

552 the water utility to progressively subdivide a large distribution system into sub-zones, without  
553 decreasing the overall performance of the network or altering the sources milking, but  
554 obtaining in this way a more reliable and controllable system, possibly enhancing the quality  
555 of delivered water and a reducing the risk of contaminant spread.

556 The methodology developed is applied to a case study in order to test its applicability  
557 and effectiveness. Results highlighted how the methodology successfully provides for the  
558 division of the water distribution network into a number of elementary districts, equal to the  
559 number of sources present in the system, that are characterized by the desired properties:  
560 connected with at least a water source, hydraulic independence from each other (i.e., no flow  
561 paths are available between two districts), and hydraulic feasibility, meaning that pressure  
562 requirements are satisfied.

563 The procedure is fast for computational time, even when elevated number of sources is  
564 elevated. Especially the union of districts phase is fast and flexible and can be potentially be  
565 used in real time application when dynamic districts are considered.

566 Further improvements of the methodology could include district boundaries  
567 optimization, considering as selection factors of the best layout also functional and non-  
568 functional constraints, such as water quality issues, administrative patrols, homogeneity of user  
569 typologies. On a different perspective, elementary districts, being from the functional point of  
570 view nothing but influence areas of sources, they could be exploited to address energy  
571 optimization problems.

572

573

## 574 **REFERENCES**

575 Alvisi, S., and Franchini, M. (2014). "A procedure for the design of district metered areas in  
576 water distribution systems". *Procedia Engineering*, 70, 41–50.



577 Awumah, K., Goulter, I., and Bhatt, S. K. (1991). "Entropy Based Redundancy Measures in  
578 Water Distribution Networks". *Journal of Hydraulic Engineering*, 117(5), 595–614.

579 Baker, M. (2009). "The Baker report: Municipal water distribution system security study-  
580 recommendations for science and technology investments". U.S. Dept. of Homeland  
581 Security, Washington, DC.

582 Butler, D., (2000). "Leakage Detection and Management". *Palmer Environmental Ltd*,  
583 Cwambran, UK.

584 Charalambous, B. (2005). "Experiences in DMA redesign at the Water Board of Lemesos,  
585 Cyprus". *Proc. IWA Specialised Conf. Leakage*, IWA Publishing, Halifax, NS.

586 Coelho, S. T. (1997). "Performance in water distribution: a systems approach". *Water  
587 engineering and Management*, Research Study Press Ltd. .Somerset, England, ISBN 0  
588 86380 219 2 , pp.222.

589 Cormen, T., Leiserson, C., Rivest, R., and Stein, C. (2009). "Introduction to Algorithms". MIT  
590 Press, Cambridge, MA.

591 De Paola, F., Fontana, N., Galdiero, E., Giugni, M., Savic, D., Sorgenti degli Uberti, G. (2014).  
592 "Automatic multi-objective sectorization of a water distribution network". *Procedia  
593 Engineering*, 89, 1200-1207.

594 Diao, K., Zhou, Y., Rauch, W. (2013). Automated creation of district metered area boundaries  
595 in water distribution systems. *Journal of Water Resources Planning and Management*,  
596 139 (2), 184-190.

597 Di Nardo, A., and Di Natale, M. (2010). "A heuristic design support methodology based on  
598 graph theory for district metering of water supply networks". *Engineering Optimization*,  
599 43(2), 193–211.

600 Di Nardo, A., Di Natale, M., Santonastaso, G.F., Tzatchkov, V.G., Alcocer Yamanaka, V.H.,  
601 (2013). "Water Network Sectorization based on genetic algorithm and minimum  
602 dissipated power paths". *Water Science and Technology: Water Supply*, vol. 13, p. 951-  
603 957, ISSN: 1606-9749, doi: 10.2166/ws.2013.059

604 Di Nardo, A. , Di Natale, M., Musmarra, D., Santonastaso, G.F., Tzatchkov, V.G., Alcocer-  
605 Yamanaka, V.H., (2015). "Dual-use value of network partitioning for water system  
606 management and protection from malicious contamination", *Journal of Hydroinformatic*,  
607 17(3) 361–376.

608 Dijkstra, E. W., (1959). "A note on two problems in connection with graphs". *Numerische*  
609 *Mathematik*, 1, 269–271. <http://doi.org/10.1007/BF01386390>

610 Farley, M. R., (1985). "District Metering". Part I - system design and installation. WRc,  
611 Swindon, U.K.

612 Ferrari, G., Savic, D., and Becciu, G., (2013). "A Graph Theoretic Approach and Sound  
613 Engineering Principles for Design of District Metered Areas". *Journal of Water Resources*  
614 *Planning and Management*, 140, 1–13. [http://doi.org/10.1061/\(ASCE\)WR.1943-](http://doi.org/10.1061/(ASCE)WR.1943-)  
615 [5452.0000424](http://doi.org/10.1061/(ASCE)WR.1943-5452.0000424)

616 Giustolisi, O., Kapelan, Z., and Savic, D. A., (2008b). "An algorithm for automatic detection  
617 of topological changes in water distribution networks", *Urban Water*, 7(1), 1-15.

618 Giustolisi, O., Ridolfi, L. (2014). "New Modularity-Based Approach to Segmentation of Water  
619 Distribution Networks". *Journal of Hydraulic Engineering*, 140 (10), 04014049.

620 Grayman, W. M., Murray, R., Savic, D. A., (2009). "Effects of redesign of water systems for  
621 security and water quality actors", *Proc. of the World Environmental and Water Resources*  
622 *Congress*, Kansas City, MO.